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AIR-WATER FLOW IN EVORAULIC STRUCTURES

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UNITED STATES DEPARTMENT OF THE INTERIOR WATER AND POWER RESOURCES SERVICE As the Nation's principal conservation agency, the Department of the Interior has the responsibility for most of our nationally owned public lands and natural resources, protecting our fish and wildlife, preserving the environmental and cultural values of our national parks and historical places, and providing for the enjoyment of life through outdoor recreation. The Department assesses our energy and mineral interests of all our people. The Department also has a major responsibility for American Indian reservation communities and for people who live in Island Territories under U.S. administration.

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Preface

The material assembled in this report is the result of studies extending over many years by a large number of engineers. Ellis Picket at the U.S. Army Engineer Waterways Experiment Station in Vicksburg, Mississippi, supplied a reference list dealing with air-water problems. Personnel of the Water and Power Resources Service E&R Center. Water Conveyance Branch made their files and drawing on air design criteria in pipelines available for publication in this report. Prior to publication, the report was reviewed by Ellis Pickett and Ted Albrecht with the U.S. Army Engineers; and by engineers in the Dams, Mechanical, and Water Conveyance Branches, E&R Center, Water and Power Resources Service. The many constructive comments by these individuals and the assistance of Richard Walters who provided continuity and technical editing is greatly appreciated.

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Letter Symbols and Quantities

Symbol	Quantity	Symbol	Quantity
Δ	Cross sectional area of water prism	d	Flow depth
4	Cross sectional area of airflow	d_b	Bulked flow depth
<i>л</i> а а.	nassage	d_{e}	Deflector height
1	Cross sectional area of air core in a	d_n	Nappe thickness
Ac	vertical shaft	d_o	Orifice diameter
4,	Cross sectional area of conduit	d_t	Total depth of underlying and air
1	Orifice area		free zones
А. А	Cross sectional area of penstock	d_{95}	Bubble diameter for which 95
Ар Д	Cross sectional area of vent		percent of the air, by volume, is
л _у 9	Ratio of bubble terminal velocity in		contained in bubbles of this
μ. μ	turbulent flow to terminal velocity		diameter or smaller
	in still water	${oldsymbol E}$	Relative width of the frequency
ρ.	Mean air distribution function		spectrum
ац Я.	Mean air distribution constant	exp	Napierian logarithm equal to
B.	Width of rectangular chute	-	2.71828, approximately
Б	Width of flow channel	f	Darcy-Weisbach friction factor
Ь	Nappe width	Ġ	Gate opening
b	Empirical coefficient accounting for	G_{g}	Mass velocity of gas
Us	sand grain roughness	Ğĭ	Mass velocity of liquid
С	Air concentration	g	Gravitational constant (acceleration)
Č.	Actual air concentration	\dot{H}	Hydraulic radius of prototype air
C_{μ}	Drag coefficient on a bubble		vent
	Discharge coefficient based on 100	H_{f}	Fall height of a water jet
σa	percent gate opening	H_m	Head across orifice
Cr.	Local loss coefficient	H_n	Net head across turbine
C_{I}	Air concentration at $d_t/2$	Ho	Distance from channel invert to
C_	Air concentration measured by a	•	energy grade line
~m	pitot tube sampler	H_t	Total potential and kinetic energy
C.	Orifice discharge coefficient	h	Mean wave height
C.	Drag coefficient on a sphere	h _a	Height of airflow passage
Ċ,	Air concentration at the bottom of	h _f	Distance from inlet to the water
-1	the mixing zone		level in the vertical shaft
\overline{C}	Mean air concentration	h_l	Head loss per unit length
с	Waterhammer wave celerity	h_m	Head across manometer
D	Conduit diameter	h_w	Allowable head rise in pensiock
D_{h}	Smaller dimension of a rectangular	K_e	Entrance loss
U	conduit	K _s	Singular (form) loss
D_d	Diameter of water drop	$m{k}$.	Von Karman universal constant
D.	Equivalent bubble diameter	· _	equal to U.4
D_{s}	Larger dimension of a rectangular	k_r	Coefficient of roughness
•	conduit	ks	Sand grain roughness

LETTER SYMBOLS and QUANTITIES—Continued

Symbol	Quantity	÷.,	Symbol	Quantity
L	Length of conduit or vent		r _s	Relative roughness of conduit
L_c	Distance to start of self-aeration			(rugosity to diameter ratio)
L_r	Prototype to model scale ratio		\boldsymbol{S}	Submergence depth
L_s	Distance between stiffener rings		S_o	Pipe slope
M	Unit mass		S_f	Slope of energy grade line
M_o	Maximum difference in elevation		s	Root-mean-square value of wave
•	between a wave crest and the			height distribution
	mean water level		Sw.	Root-mean-square value of water
m	Air concentration distribution			surface distribution
	coefficient		T	Top width of flow passage
N	Safety factor		`t	Pipe wall thickness
, n	Manning's roughness coefficient		${oldsymbol{U}}$	Free stream velocity
n_v	Velocity distribution power-law		U_d	Velocity of water drop relative to
D	Energy dissipated			
r D	Normal distribution function		U_j	water jet velocity
D.	Drobability that the ways baight is		u V	Moon flow valuation
⊥ h	riobability that the wave height is		V V	Tomainal indexity of hashblas
D	Drobability that the water surface		₽f	in turbulant flow
ſw	is actual to an empeter than the		τź	In turbulent now
	is equal to or greater than the		V _i V	Minimum and a its available
•	Breesen interview		$V_{\rm m}$	Winnum velocity required to
p	Allevenha intensity		17	entrain air
_ p_a	Allowable internal pressure		V _o	Maximum water surface velocity
Patm	Atmospheric pressure		V _s	I erminal velocity of bubbles in
p_c			T/	sing flow
Pin	Neger and pressure		r _t	reminal velocity of buddles in
p_n	Discharge		177	
Ŷ	Discharge		W	wetted perimeter
Q.	Volume Howrate of air		x	Distance from start of boundary
V.	Critical discharge			layer growth
Qr	Discharge from reservoir		У	Distance normal to channel bottom
Qw.	Volume Howrate of water			(flow depth)
q	Unit discharge	;	У <u>в</u>	Distance from water surface
q.	Insultation rate of air per unit		yc '	Conjugate depth
מ	Surface area		Ye	Effective depth
л р	Buddle radius		\boldsymbol{y}_{k}	Critical depth
К _b р	Equivalent bubble radius		y'	Normal distance to the bottom of
K ^c	nadius of curvature of the bubble			the mixing zone
מ			z	Elevation
к _ј	I DICKNESS OF annular jet			
r	Water jet radius			

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Symb	ol	Quantity	Symbo	l Quantity		·
α	alpha	Angle chute invert makes with horizontal	Ε	Eötvös number	=	$\frac{\gamma D^2}{\sigma}$
β	beta	Ratio of volumetric airflow rate to waterflow rate	\mathbf{E}_{u}	Euler number	=	Δp $e^{1/2}$
γ	gamma	Specific force of water				12
ర	delta	Boundary layer thickness	F	Froude number	=	1 + 1 + 1 + 1 + 2 = 1 + 1 + 1 + 1 + 2 = 1 + 1 + 1 + 1 + 2 = 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1
£	epsilon	Mass transfer coefficient of bubbles	Р	Prandtl velocity		(gr)
ζ	zeta	Air concentration distribution constant		ratio	-	$\frac{1}{(1_0/q)^{1/2}}$
η.	eta	Normalized wave height	п			h " Idn/de
θ	theta	Void fraction	\mathbf{P}_{θ}	Poiseuille number	=	v II
x	kappa	Gas constant				- µ <i>v</i>
λ	lambda	Density ratio	R	Revnolds number	=	ΓD
· μ	mu	Dynamic viscosity		Iteynoide nameer		ν
μ_{a}		Dynamic viscosity of air				
μ_{w}		Dynamic viscosity of water	R _x	Distance Reynolds		V_{τ}
ν	nu	Kinematic viscosity		number	=	<u>, </u>
ν_f		Water viscosity				·
π	pi	Ratio of the circumference	W	Weber number	=	$\boldsymbol{\nu}$.
		of any circle to its	**	Weber Humber		(o/QD) ¹⁷²
		radius, 3.14159				
ę	rho	Density				
Qa		Air density			•	
Qw		Water density				
Qg		Gas density	•	· .		
QI		Liquid density				
Qm		Density of manometer fluid				
σ	sigma	Interfacial surface tension	•			
τ0	tau	Wall shear stress				
τj		Shear stress at water jet				
U _{atm}	upsilon	Specific volume of air at	•			
		atmospheric pressure				
υ *	•	Shear velocity				
ψ	psi	Multicomponent flow parameter			•	
ω	omega	Volume of gas bubble				
ω _a	-	Volume of air	· .			
ωw		Volume of water				
				. 		

LETTER SYMBOLS and QUANTITIES—Continued

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Introduction

In many engineering projects a strong interaction developes between the water flowing through a structure and the air which is adjacent to the moving water. Sometimes the interaction produces beneficial effects. However, more often than not, the effects are not beneficial and the remedial action required to reduce the effects can be costly.

Cases in which air-water interaction develop include:

- Open channels with fast flowing water that require depths adequate to contain the air which is entrained within the water
- Morning-glory spillways that must have a capacity to convey the design flood and its entrained air
- Vertical shafts that entrain large quantities of air at small water discharges
- Measuring weirs that need adequate ventilation to prevent false readings and to eliminate surging
- Outlet gates that require adequate aeration to prevent the development of low pressures—which can lead to cavitation damage
- Emergency gates at penstock entrances that require ventilation to prevent excessive negative internal pressures during draining or emergency gate closures

- Sag pipes (inverted siphons)¹ that can be damaged due to blowback of entrained air
- Long pipelines that require air release and vacuum relief valves

From these cases it is noted that air-water flows can be generalized into three basic flow types:

1. Air-water flows in open channels,

2. Air-water flows in closed conduits, and

3. Free-fall water flows.

The first type usually is called *air-entraining* flow because air is entrained into the water mass. The second basic flow type generally is referred to as *air-demand*. The term *airdemand* is both misleading and technically incorrect, since an air vent does not demand air any more than an open valve demands water. However, since the term has been in common use for over 20 years, efforts to improve the nomenclature seem rather futile. The third type is referred also to as *air-entraining flow*.

[&]quot;siphon, inverted—A pipe line crossing over a depression or under a highway, railroad, canal, etc. The term is common but inappropriate, as no siphonic action is involved. The suggested term, sag pipe, is very expressive and appropriate." Nomenclature for Hydraulics, Comm. on Hyd. Str., Hyd. Div., ASCE, 1962.

Purpose and Application

The purpose of this report is to summarize the work that has been done on *air-entrainment* and *air-demand* regarding the most recent theories and to suggest ways in which the results can be applied to design. The intent was to produce a concise reference of material from which design manuals, nomographs, and charts for specific applications could be prepared.

Although many generalizations of the data can be made, some types of flow conditions that are encountered in practice can be treated only by individual studies with physical models. These cases are identified when they occur.

Additional studies are needed in many areas. Some of the most critical areas requiring further research include the following:

- Effects of turbulence and air concentration on bubble dynamics
- Fluid dynamics in the developing aeration regime of free-surface flow
- Effects of hydraulic and conduit properties on probabilistic description of water surface in free-surface, high-velocity flow
- Effect of pressure gradients on air flow in partially-filled, closed conduits
- Bubble motion in closed-conduit flows for conduit slopes exceeding 45-degrees
- Effects of ambient pressure levels on cavitation characteristics of gates and valves discharging into a closed conduit
- Interaction between the air and a free jet

Summary and Conclusions

Methods have been developed to predict the mean air concentration and the concentration distribution with open channel flow. <u>A new</u> description of the free water surface in high velocity flow is proposed which more accurately represents actual conditions in high velocity flow. The effect of air entrainment on the performance of a stilling basin can be estimated using a bulked flow concept. A computer program (app. II) is presented with which the mean air concentration in steep chutes and spillways can be estimated.

With exception of a falling-water surface and decreasing flow in pipelines, closed conduit flows require model studies. When properly conducted and analyzed, model studies will yield accurate data for estimating air-flow rates. Experimental methods are discussed. A computer program (app. III) is presented which can be used to predict the airflow rate with a falling-water surface. Design charts are presented for sizing air relief valves and vacuum valves on pipelines.

The airflow rate in vertical shafts was found to be extremely dependent upon the flow conditions at the shaft inlet. Equations are included for estimating the airflow rate having various inlet conditions.

Factors influencing the airflow rate around free falling jets are discussed. This area is identified as one needing additional research. Equations are presented from which the air entraining characteristics of a jet entering a pool can be estimated.