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SUSITNA HYDROELECTRIC PROJECT

REGIONAL FLOOD STUDIES

DECEMBER 1981

Prepared by:



Prepared for:



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ALASKA POWER AUTHORITY SUSITNA HYDROELECTRIC PROJECT

TASK 3 - HYDROLOGY

REGIONAL FLOOD STUDIES

DECEMBER 1981

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REGIONAL FLOOD STUDIES

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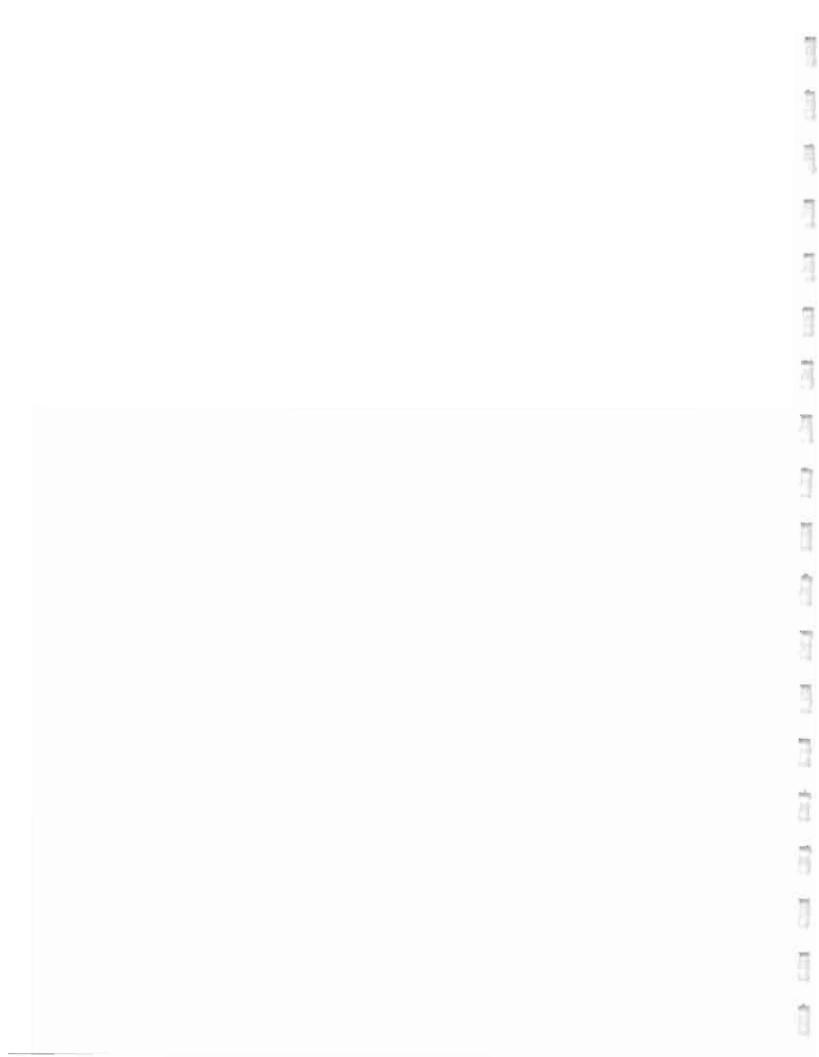
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1 - INTRODUCTION

On January 1, 1980, Acres American, Inc., received notice to proceed with the feasibility analysis of the Susitna Hydroelectric Project. The analysis will consider the feasibility of constructing one or possibly two dams along the upper Susitna River. The two primary sites being considered are the Watana and the Devil Canyon sites.

The objective of this study is to provide design flood peak information for the design of the project and for assessing pre-project flood conditions in the Susitna River reaches located downstream and upstream from the proposed Watana and Devil Canyon dam sites. Within this context, two types of floods were studied: the largest annual floods and the largest annual floods during ice conditions (October-May). Procedures were developed to estimate the annual instantaneous peak and the October-May instantaneous peak for selected frequencies of occurrence on ungaged rivers within the upper Susitna River basin. Procedures were also developed to estimate the error associated with estimates made by the above mentioned procedures. Typical hydrographs were developed indicating flood shape, peak, and volume for of frequencies occurrence. Flow volume-duration frequency curves were also developed for the May-July and August-October periods on the Susitna River at Gold Creek.



2 - SUMMARY

This report describes the Susitna Regional Flood Peak Frequency Analysis conducted by R&M Consultants, Inc., for Acres American, Inc. Acres American, under contract with the State of Alaska, is conducting a study concerning the feasibility of developing a hydroelectric complex on the upper Susitna River.

The results of single station flood frequency analyses are presented for 12 stations having annual instantaneous flood peak data, and for 11 stations having maximum October-May mean daily flow data. Four frequency distributions were analyzed, from which the Three Parameter Log Normal Distribution was selected. Subsequently, the October-May maximum mean daily flows for selected frequencies of occurrence were converted to instantaneous peaks.

By using the index method of regionalizing the data, procedures were developed to estimate the annual instantaneous peak and the October-May instantaneous peak for selected frequencies of occurrence on ungaged rivers within the upper Susitna River basin. Procedures were also developed to estimate the error associated with estimates made by the above mentioned procedures. The results of this analysis are then compared to those of previous regional analyses.

Typical hydrographs for different frequency floods were developed for the Susitna River at Gold Creek. A flood volume-duration analysis was also conducted for May-July flows recorded at Susitna River at Gold Creek.



3 - FLOOD FREQUENCY ANALYSIS

The Susitna Regional Flood Frequency Analysis was conducted in the following four steps;

- a. Single station flood frequency analyses were conducted at stations thought to be hydrologically similar to the upper Susitna River basin.
- b. The single station flood frequency analyses were regionalized.
- c. An error analysis was conducted in order to estimate the amount of error associated with flood peak estimates in ungaged areas.
- d. The results of this analysis were compared with 2 regional analyses which have been conducted by the U.S. Geological Survey.

Each of these steps is explained fully in the report that follows:

3.1 - Single Station Frequency Analysis

In order to select stream gaging stations that are hydrologically similar to the upper Susitna River basin, factors such as mean annual runoff volume, climate, and geology were examined for rivers with USGS stream gaging stations in southcentral Alaska. Of those stations identified, only those with 10 or more years of record and greater than 150 square miles in size were utilized. The basins finally selected were as follows:

Susitna River at Gold Creek
Susitna River near Denali
Maclaren River near Paxson
Susitna River near Cantwell
Chulitna River near Talkeetna
Talkeetna River near Talkeetna
Montana Creek near Montana
Skwentna River near Skwentna
Caribou Creek near Sutton
Matanuska River at Palmer
Tonsina River at Tonsina
Copper River near Chitina

The last four stations are not as good as the first eight, due to their distance from the Susitna basin and their exposure to a maritime climate. However, these differences were not sufficient to warrant exclusion from the analysis. Three types of floods were abstracted from the records: annual peak annual discharge, maximum discharges, and annual maximum October-May mean discharges. Both the annual instantaneous peak discharge and the mean daily discharge data are available for each of the stations selected, with the exception of Montana Creek. Montana Creek is a crest gage station with only annual instantaneous peak discharge data available.

For each station with mean dally discharge data, a ratio was developed between the annual instantaneous peak discharge and the annual maximum mean daily discharge (Table 3.1). Where an occasional annual instantaneous peak discharge value was missing from the station record, the ratio was used to predict the annual instantaneous peak value.

A Fortran IV computer program (Condie et. al., 1977) was used to fit the Gumbel I, Log Normal, Three Parameter Log Normal and Log Pearson Type III distributions to both the annual instantaneous flood peaks and the annual maximum October-May mean daily discharges. The program ranks the annual flood peaks and fits the frequency curves by the method of maximum likelihood. When the maximum likelihood methods have no true solution, a moments fit is used. For the Log Pearson Type III distribution, both the maximum likelihood and the moment fits are calculated in the program. The ranked discharges are assigned plotting positions by the Weibull formula:

$$T = (N + 1) / M$$
 (3.1)

Where:

T = return period in years

N = record length in years

M = the rank

Tables 3.2 and 3.3 illustrate the degree of fit achieved by fitting each of the distributions to both the annual instantaneous flood peak series and the annual maximum October-May mean daily discharge series. From the results shown in Table 3.3, the Three Parameter Log Normal Distribution provides the best fit of the data, although the Log Normal Distribution is also acceptable. The Three Parameter Log Normal Distribution is adopted almost exclusively throughout this study. At two stations in the annual maximum October-May mean daily discharge series it was not possible to fit the Three Parameter Log Normal Distribution to the data. At these two stations, Susitna River at Cantwell and Skwentna River near Skwentna, the Log Normal Distribution was

used. Plots of the flood frequency curves for each of the stations and flood series are exhibited in Attachments A and B.

Since the computer program only considers recurrence intervals of 1.005, 1.05, 1.25, 2, 5, 10, 20, 50, 100, 200 and 500 years, the graphs of the annual instantaneous flood peak frequency curves were extrapolated to the 10,000-year recurrence interval.

The annual maximum October-May mean daily discharge frequency curves were converted to October-May instantaneous peak discharge frequency curves using the ratios discussed earlier. Thus, the remainder of this study only considers the annual instantaneous flood peak frequencies and the October-May instantaneous flood peak frequencies.

3.2 - Regionalization of Frequency Analyses

Regionalization of the single-station frequency curves involves developing regional relationships which can be used to calculate design flood peaks within a "homogeneous flood region". Theoretically, the flood response of all catchments within a homogeneous flood region is the same. In practice, however, the term "homogeneous" implies that the response characteristics of the individual basins are not significantly different and that these differences, coupled with sampling errors associated with the discharge measurement, yield errors in the regional relationships which are within acceptable limits. The purpose of developing the regional relationships is to apply them to ungaged drainage basins as well as to gaged drainage basins for which discharge records are too short to yield accurate results.

To determine if the stations selected were homogeneous with regard to runoff, a standard Student's "t" test of homogeneity of the $Q_{20}/Q_2 = Y_{20}$ variables was used. For each flood series within the region being analyzed, individual station Y_{20} values were computed (Tables 3.4 and 3.5) and were used to calculate the allowable range in Y_{20} by applying the following equation:

$$\bar{Y}_{20} - (t_{m-1}) \left[S_{20}^2 \left(1 + \frac{1}{(m-1)} \right) \right]^{\frac{1}{2}} < Y_{20} < \bar{Y}_{20} + (t_{m-1}) \left[S_{20}^2 \left(1 + \frac{1}{(m-1)} \right) \right]^{\frac{1}{2}}$$
 (3.2)

Where:

Y₂₀ = ratio of 20-year return flood to the mean annual flood for a single station

 \bar{Y}_{20} = an average of the single station Y_{20} values for a particular flood series

t_{m-1} = "t" statistic for m-1 degrees of freedom at
the 95 percent confidence level

S₂₀ = standard deviation of the individual stations at the 20-year return period

m = number of stations in the region

As can be seen from Tables 3.4 and 3.5, all individual Y_{20} value are within the limits of the 95% confidence level. Therefore, all of the stations selected for use in the Susitna Regional Flood Frequency Analysis are homogeneous with respect to both annual instantaneous peak discharges and October-May instantaneous peak discharges.

Once it was determined that all of the stations selected for use in the analysis were homogeneous with respect to discharge, the index method was used to regionalize the single-station data. The index method consists of two parts (Dalrymple, 1960). The first involves the development of a regional dimensionless frequency curve representing the ratio of the flood of any given frequency to the mean annual flood. The second part requires the development of regression equations for relating the mean annual flood to the physiographic and climatic characteristics of the drainage basins. By combining the mean annual flood with the regional dimensionless frequency curve, a frequency curve for any ungaged location within the region can be developed.

This method assumes that throughout large regions that are considered to be homogeneous with respect to flood producing characteristics, individual basins with widely varying drainage areas will have frequency curves of equal shape and slope. Any differences in the shape and slopes of the individual curves are attributed to sampling error.

To produce a regional curve from single station curves, Dalrymple (1960) recommends that the dimensionless frequency curves for a homogeneous region be evaluated as the median of the individual frequency curves within the region. Since the weighted median has also been found to be useful in this type of analysis (Acres American, 1977), the dimensionless discharges at the 1.005, 1.05, 1.25, 2, 5, 10, 20, 50, 100, 200 and 500-year recurrence intervals were analyzed with regards to the mean, median and weighted median at each recurrence interval (Tables 3.6 and 3.7). weighted median was determined by using the number of years of record at a station as the weight for that station. The dimensionless frequency curve was estimated from the median of the 12 stations used in the analysis. The median value had a better general fit than either the mean or the weighted median values. The mean value at the 10,000-year return period was abnormally high, while the weighted median values started decreasing after the 100-year return period.

The confidence limits for each of the regional curves are based on values computed at the 1.005, 1.05, 1.25, 2, 5, 10, 20, 50, 100, 200 and 500-year recurrence intervals from the standard deviation of the single station curves about the regional curves, using the following equations.

The standard deviation of single station curve values about the regional curve is given by

ST =
$$\begin{bmatrix} \frac{m}{\sum} [(Y_T - \bar{Y}_T)]^2 \\ \frac{1}{(m-1)} \end{bmatrix}^{\frac{1}{2}}$$
 (3.3)

Where:

ST = standard deviation at T-year return period

 \bar{Y}_T = value of median dimensionless regional curve at T-year return period

Y_T = value of dimensionless single station curve at T-year return period

m = number of single station curves

The standard error of the median (or regional curve in this case) as a proportion of the median is given by

$$SEM_{T} = \frac{ST}{\bar{Y}_{T} (m)^{\frac{1}{2}}}$$
 (3.4)

Where:

SEM_m = relative standard error i median, i.e. regional curve at T-year period.

The associated 95 percent confidence limits for the regional curve are derived from the standard error and the corresponding "t" statistic coefficients.

A stepwise multiple linear regression computer al., 1977) was used to relate the mean annual flow and the mean October-May instantaneous physiographic and climatic characteristics of the The program performs a forward stepping regression analysis and tests the "significance" those used in the equation and those remaini: their "F" ratio at each step of the analysis. of the estimate and the coefficient of determina on are determined for each step. The physiographic and climat and their values for each of the stations Table 3.8.

rogram (Dixon et. istantaneous peak peak flow to the drainage basins. multiple linear of variables, both to be used, by ne standard error parameters used are presented in

In order to test the effect of a log-transform or HEC Multiple Linear Regression computer p Non-transform and log-transform regressic conducted on the mean annual instantaneous mean October-May instantaneous peak flow. T > non-transformed results obtained by HEC's stepwise backward results obtained in the initial analysis. The f indicates that the log-transform analysis die Consequently, the non-transform r pression equations statistics. were selected.

the flow data, the gram was used. analyses were eak flow and the rogram duplicated lowing information not improve the

	Standard Er of Estimate (or <u>5)</u>	95% Error
Mean Annual Flood					
Non-Transform Log-Transform	0.9994 0.9930	+1465 +2612	-1: -2:	5 2	4% 14.5%
Oct-May Mean Flood					
Non-Transform Log-Transform	0.9709 0.9141	+3081 +4425	-36 -25	100	20% 50%

The equations that were finally selected, including the coefficient of determination and the standard error of the estimate, are as follows:

Mean Annual Instantaneous Flood Peak

$$Q_A$$
 = 7.06(D.A) + 46.36(L) + 697.14(G)
+ 200.15(MAP) - 49.55(MAS) - 2594 (3.5)
 R^2 = 0.9994

S.E.E. = 1465

Mean October-May Instantaneous Flood Peak

$$Q_{o-m} = 1.56(D.A.) + 143.35(L) - 2894$$
 (3.6)

 $R^2 = 0.9853$ S.E.E. = 3081

Where:

Q_A = Mean Annual Instantaneous Flood Peak, cfs.

Q_{n-m} = Mean October-May Instantaneous Flood Peak, cfs.

D.A. = Drainage Area, sq. mi.

L = Stream Length, mi.

G = Percent of Drainage Area Covered by Glaciers, %

MAP = Mean Annual Precipitation, in.

MAS = Mean Annual Snowfall, in.

Of the five parameters selected for computing the mean annual instantaneous flood peaks, drainage area explains over 98 percent of the variation between the regional mean annual flood and those of the individual stations. However, inclusion of the other four basin and climatic parameters (main channel length, percent of glacial area, mean annual precipitation, and mean annual snowfall) significantly reduced the standard error of estimate for the mean

annual flood peak. Since the regional mean annual flood is the basis from which all other recurrence intervals are estimated, the four other parameters were included.

Similar logic applies to the selection of parameters for the mean October-May instantaneous flood peak. Drainage area and main channel length explain over 97 percent of the variation. However, inclusion of additional parameters did not significantly decrease the standard error of estimate in this case, so only the above two parameters were used.

3.3 - Relative Standard Error of the Estimate for the Index Method

Equation (3.3) gives the standard deviation, $S_{\rm T}$, of the dimensionless frequency curves about the regional curve. Equation (3.7) is then used to calculate the associated relative standard error of estimate, $SE_{\rm T}$.

$$SE_{T} = \frac{S_{T}}{\tilde{Y}_{T}}$$
 (3.7)

Where:

 $\mathbf{\tilde{Y}}_{\mathbf{T}}$ = value of median dimensionless regional curve for the T-year return period

SEM_T in Equation (3.4) represents the standard error of estimate of the median or regional curve. SE_T in Equation (3.7) represents the standard error of the individual station curves about the regional curve. Note that SEM_T = SE_T / (m)².

In order to combine the errors SE_T associated with the application of the index curve with the errors in the calculation of the mean annual flood, Equation (3.8) is applied. An outline of the derivation of this equation is presented in the report entitled "Regional Flood Frequency Analysis" (Acres American, 1977).

$$SEC_{T} = \begin{pmatrix} 2 & 2 & \frac{1}{2} \\ SE_{T} + SE_{2} \end{pmatrix}$$
 (3.8)

Where:

 SEC_T = composite relative standard error of estimate of T-year event

SE₂ = relative standard error of estimate associated with the evaluation of the mean annual flood

Figures 3.1 and 3.2 present dimensionless flood frequency curves which can be used in conjunction with Equations (3.5) and (3.6) to predict the annual instantaneous and October-May instantaneous flood peaks at specific recurrence intervals. The figures also present curves for computing the associated composite 95 percent confidence limits for the estimate.

3.4 - Comparison with Results of Previous Studies

Previous regional flood studies carried out in the Susitna River basin and surrounding basins include:

(a) Lamke, R.D. (1979)

Multiple regression equations for different recurrence intervals were developed for two regions of the state. Area I consists of most areas of Alaska with a maritime climate, excluding the Aleutian Islands and the Pacific Ocean side of the Alaska Peninsula. Area II consists of those parts of Alaska with transitional, continental, and arctic climates, together with the Aleutian Islands and the Pacific Ocean side of the Alaskan Peninsula which have maritime climate. The Susitna River basin is included in Area II.

(b) Freethey, G.W. and D.R. Scully (1980)

Multiple regression equations for different recurrence intervals were developed using peak discharges from 50 Cook Inlet gaging stations with 10 years or more of data.

No regional flood studies in the area have used the index method.

Table 3.9 lists the parameters used in the regression analyses and compares the standard errors of estimate for the 2-, 5-, 10-, 50- and 100-year flood peaks. Values for the 100-year flood peaks were not derived by either of the USGS studies. It is evident from this table that the standard errors achieved in this study are lower than those in the other studies. Fewer stations were used in this study, but the stations were either within the transitional climate region or were on its borders. The USGS studies used gaging stations over much larger areas, with consequently larger differences in climate. Attachment C presents a comparison between the Susitna Regional Flood Peak estimates and estimates using the two USGS methods.

TABLE 3.1

RATIO OF INSTANTANEOUS FLOOD PEAKS TO MAXIMUM MEAN DAILY FLOW FOR SPECIFIC STATIONS

Station Name	Location	QP/QD ¹	Standard <u>Deviation</u>	Number in Sample
Susitna River	at Gold Creek	1.060	0.036	31
Caribou Creek	near Sutton	1.617	0.365	22
Matanuska River	at Palmer	1.221	0.168	24
Susitna River	near Denali	1.104	0.049	19
Maclaren River	near Paxson	1.091	0.077	19
Susitna River	near Cantwell	1.082	0.059	6
Chulitna River	near Talkeetna	1.044	0.032	13
Talkeetna River	near Talkeetna	1.219	0.102	16
Skwentna River	near Skwentna	1.067	0.044	18
Tonsina River	at Tonsina	1.031	0.036	28
Copper River	near Chitina	1.043	0.061	23

Mean Ratio of Annual Instantaneous Flood Peak to Annual Maximum Mean Daily Flow.

TABLE 3.2

DISTRIBUTION STATISTICS FOR ANNUAL INSTANTANEOUS PEAK AND OCT-MAY MEAN DAILY PEAK FLOWS

	×.					GUZUSEL I			Foc	NORMAL			3	PARAMETER LOG N	OKYAL		<u>L0x</u>	(HOEERIA)	I	(m)	Y PEARSON TYPE 1 iximum likelihood	ir.
STATION	RIVER NAME	105×116N	Ħ,	<u>cs</u>	<u>α</u>	SUM OF DEVIATIONS	MEAN DEVIATION	CSL	CKL	SUM OF DEVIATIONS	MEAN DEVIATION	CSLA	CKLA	AVG.CV.	SUM OF DEVIATIONS	MEAN DEVIATION	AVG.CV.	SEN OF DEVIATIONS	HEAM DEVIATION	AVG.CV.	SUM OF DEVENTIONS	DEVIATIONS
ANNUAL	INSTANTAN	EOUS PEAKS	FLO	S											1.0			10.0	3.97	15.51	-20.67	4.38
3920	Sustena 2.	a: Gold Cr.	25	0.6810	2.9269	-22.90	5.16	2.0564*	2.8162	-17.28	4.23	-0.0805	2.9658	15.01*	-18.09*	4.11*	15.39	-19.0z	3.97	-01/1		3
2820	Carthou Cr.	nr. Sutton	21	0.9338	4.2274	-22.03	7.14	-0.04°)	3.6494	-19.39*	7.07*	-0.0228*	3.6401*	15,534	-19.5€	7.11	16.49	-19.57	7.15	16.16	-21.98	7.31
2840	Matanuska R.	at Palmer	17	0.2193	3.6631	10.05	4.32	-9.5370	1.9170	5.46	4.17*	-0.0121*	3.5957*	10,59	2.034	5.53	Uppe	r Boundary la To	o Lav	Uppe	er Boundary la To	o Les
2910	Sustina R.	ac. Depail	20	2 4985	10. 3480	-49.50	12.95	1.8453	7.0725	-33.19*	11.21	0.0613*	4,2261*	23.59*	-38.16	8.39*	35.46	-38.81	8.39*		No Solution	
		or. Passon	22	0.9067	3,4430	-15.76	7.16	0.5539	7.6687*	- 8.75	6,14	-0. 1915*	2.6565	25.66	8.93	3.99*	15.93*	- 8.13*	4.90		Ne Solution	
. 2912	Maclaren R.			1.1			€.46	-0.14/4	3.2888	1,15	5.85*	-0.1172*		18.79*	1.32	6.01	19.01	1.92	6.21	Upp	er Boundary Is To	to Low
2915	Susitna H.	ar. Contrell	11	0.3325	3.1348	- 0.90*					7				-27.36	9. 21*		c Boundary Is To	a Nich	12.824	-29.93	9.45
2924	Chultena R.	or. Talkeeins	18	2.9054	11.7449	-33.82	10.15	2.0599	10.0128	-23.88*	10.58	0.3512*		14.05						32.65	- 5.43	1.92*
2921	Talkeetna R.	n. Talkeetna	15	1.8140	6.6111	- 1.99	8.16	1.0398	4.7599	5.99	8.20	-9.0921*	4 07524	30.544	- 0.74*	5.27	40.08	- 3.38	3.65			
2928	Mantona Cr.	nr. Hoptana	10	0.9133	6.6962*	-26.31	9.59*	-1.2149	7.5292	-26.37	11.51	0.0324*	5.9398	27.31	-25.50*	9.65	Uppe	r Soundary Is To	o Low		er Boundary is Ta	
2943	Skventna R.	nr. Skuencna	20	1.0748	4.6568*	-20.24	5.42	0.4530	4.0757	-19.07	6.23	0.0346*	4.2125	11.90*	-18.27*	5.42	13.81	-18.61	5.20*	12.23	-20.29	5. 53
2080	Tonsina 8.	at Tensina	28	0.6969	3.3804	-11.3)	- 3.54	0.03054	2.8533	- 9.25	3.44	-0.0817	2.8733*	12.73*	- 9.15*	3.09*	12.88	- 9.58	3.38	12.83	-(1.34	1.65
2120	Copper R.	pr. Chitina	25	1.6079	7.3832	-16,95	3.39	0.9367	5.5081	-15.77	3.71	0.3061*	4.8017*	8.59	- 15.57*	3.11-	11.98	-15.68	3.14	8.35*	-16.99	3.40
OCTMA	Y MEAN DAI	LY PEAK FL	OWS					pli '			-					. 44						70.0
2970	Sustena F.	at Cold Cr.	30	0.2895	3.8267	9.49	6.04	-0.9208	5.1200*	4.47	5.98	0.0009*	3.6135	8.76	- 3.39*	3.824	Uppe	r Boundary Is To	o Lov	Upp	er Boundary Is To	c Low
2825	Caribou Cr.	nr. Succon	23	1.18-7	4.6347	-16.66	12.90	-0.5297	4.3443	-4.40*	2.82*	0.0363#	3.4587*	22.25	- 9.95	7.51	24.25	- 9.34	7.36	20.34*	-12.39	B. 25
7849	Hatanuska R.	at Palmer	24	1.7691	8.7422	-16.47	6.90	-0.0550	3.6148*	-10.60*	5.97	~0.0449*	3.6202	19.25*	-10.70	6.95	20.73	-10.91	6.94	20.35	-13.76	6.79*
2910	100	pr. Denall	16	1.3738	5.3387*	-10.57	9.09	D. 1569	3.6358	- 9.124	7.30	-0.0939*	1.6889	25 84*	-10.81	6.56*	27.98	-10.54	6.76	27.46	-12.55	6.83
	Sustina R.				9. 2765	0.90	13.63	0.6728	4.3661	0. 51*	100	-0. 1931*		38.51*	- 6.32	5.31*	47.26	- 7.98	2.13	44.44	-11.66	6.17
2912	Meclaren R.	Dr. Passon	15	2.1811								-0, 1931-	3.0334-		****		29.00	10.07	5,54		No Solution	
2913	Susites A.	nr. Cantuell	10	-0.0092	3.7052	6.56	5.884	-0.90434	4.5940	5.33*	6.58			NO SOLUTION						14.150	2.0€	7,66
2924	Chulfena R.	nr. Talkretna	11	0.5364	3.9480	-1.16	7.93	-0.1723	3.3067	0.39*	6.28	-0.16)8*	3.3051*	24.41	0.45	6.29	24.45	1.52	6.22*	18.11*		
2927	Talkeetos A.	or. Talkertna	15	0.6151	3.1464	0.02	11.90	-0.2369	2.60704	0 14	11.01*	-0.1968*	2.6042	22.46*	0.56	11.34	22.83	1.65	11.23	Upp	er Joundary is To	io Lou
2928	Montana Cr.	nr. Montana	0**								200								× 1			
2943	Skurntna R.	nt. Storntoz	15	-0.1794	2.1487	-0.72	9.03	-0.4334	2.3244*	-0.58*	3.86			NO SOLUTION			15.68	1.10	7.93*		No Solution	
2080	Tonsina R.	at Tonsina	26	2.4301	12.2475	-1.324	10.02	0.2485	3.9367	5.01	9.98	-0.0632*	3.6:35*	21.234	7.76	\$.92	24.30	5.92	9.83	23.11	3.26	9.814
				1500	3.0599	-1.53*	3.47	-0.3625	3.0558	-5.10	3.21*	-0.028:*		9,73	-7.64	3.61	11.11	- 6.77	1.29*	7.96*	- 9.00	4.02
2120	Copper R.	ar. Chitina	21	0.2134	3.0399	~1.3J*	3.47	1 -0.70.3	7.0730	-3.20		-0.0.0.		,								

record length in years
coefficient of skewness of natural data
coefficient of Nortosis of natural data
coefficient of Skewness of the natural logs of the data
coefficient of Skewness of the natural logs of the data
coefficient of Skewness of the natural logs of the transformed data
coefficient of Skewness of the natural logs of the transformed data

Avg.CV. - average conflictent of variation for all return periods exclinated Sum of Deviations - 2.0-0₀/0₀ for 1.25, 2, 5, 10 and 20 year return periods. Hean Deviation - 510-0₀/15 for 1.25, 2, 5, 10 and 20 year return periods GI - Gumbel IN - Log Normal JUN - Three Parameter Log Normal JUN - Three Parameter Log Normal LP-M - Log Perason Type III (moments)
LP-ML - Log Perason Type III (maximum likelihood)

Distribution best fitting a given parameter
 A Crest-Gage station only
 Upper boundary is too low - Jess than twice the magnitude of the largest flood on record.
 Lower boundary is too high - greater than the smallest flood on record.

Theoretical Values
Gunbel I Log Normal
CS-1.14 CSL-0.0
CK-5.4 CXL-3.0 3PLNO CSLA-0.0 CXLA-3.0

TABLE 3.3

MEAN DISTRIBUTION STATISTICS FOR ANNUAL INSTANTANEOUS PEAK AND OCT. - MAY MAXIMUM MEAN DAILY FLOWS

ANNUAL INSTANTANEOUS PEAKS OCT. - MAY MAXIMUM DAILY FLOWS STANDARD COEFFICIENT OF STANDARD MEAN MEAN COEFFICIENT O DEVIATION VARIATION (%) DEVIATION VARIATION (%) Gumbel I 0.92 Coefficient of Skew 1.22 0.83 68 0.9199 5.86* 3.34 57 5.46* 3.20 Coefficient of Kurtosis 59 90 -2.86 8.43 -17.6415.79 Sum of Deviations 295 6.35 2.89 42 8.80 3.17 Mean Deviation 36 Log Normal 219 0.23 Coefficient of Skew 0.42 0.92 0.48 209 4.85 2,29 47 3.73 0.87 Coefficient of Kurtosis 23 --1.27* -13.5912.61 93 5.45 429 Sum of Deviations 7.21 Mean Deviation 6.31 2.95 43 2.70 37 Three Parameter Log Normal Coefficient of Skew -0.02*0.17 850 0.09* 0.08 89 1.27 4.10 31 3.41 0.40 Coefficient of Kurtosis 12 6.32 -13.34* 14.06 105 -4.45 142 Sum of Deviations Mean Deviation 5.91 2.26 38 6.79* 2.52 37 Log Pearson Type III (moments) -2.53296 Sum of Deviations -14.5811.78 81 7.50 1.74 33 7,22 2.21 Mean Deviation 5.33 31 Log Pearson Type III (maximum likelihood) -18.097.89 -7.7244 7.25 94 Sum of Deviations Mean Deviation 5.09* 2.57 50 . 7.08 1.80 25

^{*} Distribution best fitting a given parameter.

TABLE 3.4

HOMOGENEITY TEST ANNUAL INSTANTANEOUS PEAKS

Station	$Q_{20}/Q_2 = Y_{20}$
Susitna River at Gold Creek	1.83
Caribou Creek near Sutton	1.82
Matanuska River at Palmer	1.49
Susitna River near Denali	1.81
MacLaren River near Paxson	2.02
Susitna River near Cantwell	1.68
Chulitna River near Talkeetna	1.47
Talkeetna River near Talkeetna	2.33
Montana Creek near Montana	1.96
Skwentna River near Skwentna	1.49
Tonsina River at Tonsina	1.72
Copper River near Chitina	1.35
	Y 20 = 1.748
	s ₂₀ = 0.2776

Limits of 95% Confidence Interval 1.11 - 2.39

The stations selected for use in the Susitna Regional Flood Peak Frequency Analysis are homogenous with respect to the annual instantaneous peaks at the 95% confidence level.

TABLE 3.5

HOMOGENEITY TEST

OCTOBER - MAY INSTANTANEOUS PEAKS

Station	$Q_{20}/Q_2 = Y_{20}$
Susitna River at Gold Creek	1.57
Caribou Creek near Sutton	2.63
Matanuska River at Palmer	2.24
Susitna River near Denali	2.35
MacLaren River near Paxson	3.32
Susitna River near Cantwell	2.33
Chulitna River near Talkeetna	1.98
Talkeetna River near Talkeetna	2.12
Skwentna River near Skwentna	1.76
Tonsina River at Tonsins	2.45
Copper River near Chitina	1.50
	
	$\overline{Y}_{20} = 2.205$
	$s_{20} = 0.5175$

Limits of 95% Confidence Interval 0.99 - 3.41

The stations selected for use in the Susitna Regional Flood Frequency Analysis are homogenous with respect to the October - May instantaneous peaks at the 95% confidence level.

TABLE 3.6

REGIONAL ANNUAL INSTANTANEOUS
DIMENSIONLESS FLOOD VALUES

	Return <u>Period</u>	Susitna River at Gold Creek	Caribou Creek near Sutton	Matanuska River at Palmer	Susitna River near Denali	MacLaren River near Paxson	Susitna River near Cantwell	Chulitna River near Talkeetna	Talkeetna River near Talkeetna	Skwentna River near Skwentna	Tonsina River at Tonsina	Copper River near Chitina	Montana Creek near Montana	<u>Mean</u>	Median	Weighted Median
	1.005	0.50	0.45	0.45	0.81	0.79	0.49	0.76	0.62	0.66	0.53	0.76	0.17	0.58	0.52	0.58
	1.050	0.58	0.55	0.57	0.83	0.81	0.59	0.80	0.66	0.72	0.61	0.81	0.34	0.66	0.64	0.76
,	1.25	0.75	0.73	0.77	0.88	0.86	0.76	0.88	0.77	0.84	0.77	0.89	0.63	0.79	0.77	0.86
	2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1,00	1.00	1.00	1.00
	5	1.36	1.36	1.24	1.26	1.32	1.31	1.19	1.46	1.22	1,31	1.15	1.45	1.30	1.31	1.24
	10	1.60	1.59	1.38	1.51	1.63	1.50	1.33	1.86	1.36	1.52	1.25	1.72	1.52	1.52	1.69
	20	1.83	1.82	1.49	1.81	2.02	1.68	1.47	2.33	1.49	1.72	1.35	1.96	1.75	1.77	1.84
	50	2.14	2.10	1.63	2.29	2.67	1.91	1.67	3.04	1.66	1.98	1.47	2.26	2.07	2.04	179
	100	2.38	2.32	1.72	2.74	3.27	2.08	1.82	3.67	1.79	2.17	1.56	2.47	2.33	2.25	1.94
	200	2.63	2.54	1.81	3.26	3.99	2.25	1.98	4.39	1.92	2.37	1.65	2.68	2.62	2.46	2.10
	500	2.96	2.83	1.92	4.07	5.15	2.47	2.21	5.48	2.09	2.63	1.78	2.95	3.05	2.73	2.46
	10,000	3.84	3.64	1.97	9.97	11.65	3.10	2.99	11.54	2.75	3.39	2.20	3.62	5.06	3.51	3.06

3-1:

TABLE 3.7

REGIONAL OCTOBER - MAY INSTANTANEOUS DIMENSIONLESS FLOOD VALUES

Return <u>Period</u>	Susitna River at Gold Creek	Caribou Creek near Sutton	Matanuska River at Palmer	Susitna River near Denali	MacLaren River near Paxson	Susitna River near Cantwell	Chulitna River near Talkeetna	Talkeetna River near Talkeetna	Skwentna River near Skwentna	Tonsina River at Tonsina	Copper River near Chitina	Mean	<u>Median</u>	Weighted Median
1.005	0.36	0.18	0.34	0.40	0.45	0.33	0.40	0.35	0.48	0.39	0.48	0.38	0.39	0.45
1.050	0.51	0.30	0.44	0.48	0.50	0.43	0.50	0.45	0.57	0.47	0.59	0.48	0.48	0.48
1.25	0.74	0.57	0.66	0.67	0.65	0.65	0.70	0.67	0.75	0.66	0.78	0.68	0.67	0.75
2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
5	1.28	1.67	1.51	1.53	1.76	1.54	1.42	1.47	1.34	1.56	1.24	1.48	1.51	1.24
10	1.44	2.15	1.88	1.93	2.47	1.93	1.70	1.80	1.55	2.00	1.38 .	1.84	1.88	1.93
20	1.57	2.63	2.24	2.35	3.32	2.33	1.98	2.12	1.76	2.45	1.50	2.20	2.24	2.35
50	1.73	3.31	2.74	2.94	4.70	2.88	2.35	2.54	2.02	3.10	1.63	2.72	2.74	2.94
100	1.84	3.84	3.13	3.42	5.96	3.31	2.63	2.87	2.22	3.64	1.73	3.14	3.13	3.42
200	1.94	4.40	3.53	3.93	7.43	3.77	2.91	3.20	2.42	4.22	1.82	3.60	3.53	1.94
500	2.06	5.18	4.10	4.66	9.76	4.40	3.30	3.67	2.69	5.05	1.93	4.25	4.10	2.06

susi5/n1

TABLE 3.8

PHYSIOGRAPHIC AND CLIMATIC PARAMETERS¹

Station Name	Location	Drainage Area (sq. mi.)	Main Channel Slope (ft./mi.)	Stream Length (mi.)	Mean Basin Elevation (ft.)	Area of Lakes & Ponds (%)	Area of Forests (%)	Area of Glaciers	Mean Annual Precipitation (in.)	Precipitation Intensity (in.)	Mean Annual Snowfall (in.)	Mean Minimum January Temperature (°F)
Susitna R.	at Gold Creek	6,160	10.2	189.0	3420.0	1.0	7.0	5.0	29.0	2.0	200.0	-4.0
Caribou Cr.	nr. Sutton	289	13.6	30.0	4190.0	0.0	10.0	0.0	28.0	1.5	80.0	2.0
Matanuska R.	at Palmer	2,070	79.7	77.0	4000.0	0.0	14.0	12.0	35.0	1.5	80.0	4.0
Susitna R.	nr. Denali	950	56.6	51.0	4510.0	1.0	1.0	25.0	60.0	2.0	400.0	-6.0
MacLaren R.	nr. Paxson	280	133.0	23.0	4520.0	1.0	0.0	19.0	55.0	1.5	400.0	-6.0
Susitna R.	nr. Cantwell	4,140	10.0	107.0	3560.0	2.0	5.0	7.0	32.0	1.5	200.0	, -4.0
Chulitna R.	nr. Talkeetna	2,570	23.0	87.0	3760.0	1.0	22.0	27.0	55.0	1.6	250.0	-5.0
Talkeetna R.	nr. Talkeetna	2,006	35.0	90.3	3630.0	0.0	25.0	7.0	70.0	2.5	150.0	-2.0
Montana Cr.	nr. Montana	164	114.0	25.0	1930.0	3.0	. 54.0	0.0	40.0	2.2	90.0	0.0
Skwentna R.	nr. Skwentna	2,250	30.6	98.0	2810.0	5.0	34.0	16.0	43.0	2.0	140.0	-5.0
Tonsina R.	at Tonsina	420	71.0	46.0	3600.0	4.0	27.0	11.0	25.0	2.0	180.0	-2.0
Copper R.	nr. Chitina	20,600	14.4	178.0	3620.0.	3.0	22.0	17.0	37.0	2.0	120.0	-4.0

¹ Values in this table are from the report entitled "Flood Characteristics of Alaska Streams" by Lanake (1979).

TABLE 3.9 COMPARISON OF REGRESSION EQUATIONS

Study	Homogenous Flood Region	Number of Hydrometric Stations Used	Physiographic Parameters Utilized	Recurrence Interval (Years)	SE of Estimate (Percent)
Present	Susitna & Neighboring Stations	12	D.A., L, G, MAP, MAS	2 5 10 50 100	±18 ±19 ±21 ±29 ±34
(a)	Area II	163 163 132 26	D.A., MAP, LP, F, T	2 5 10 50	±77 ±78 ±79 ±68
(b)	Cook Inlet	50	D.A., LP, MAP	2 5 10 50	+56, -36 +51, -34 +52, -34 +61, -38

D.A. = drainage area, mi²
L = stream length, miles

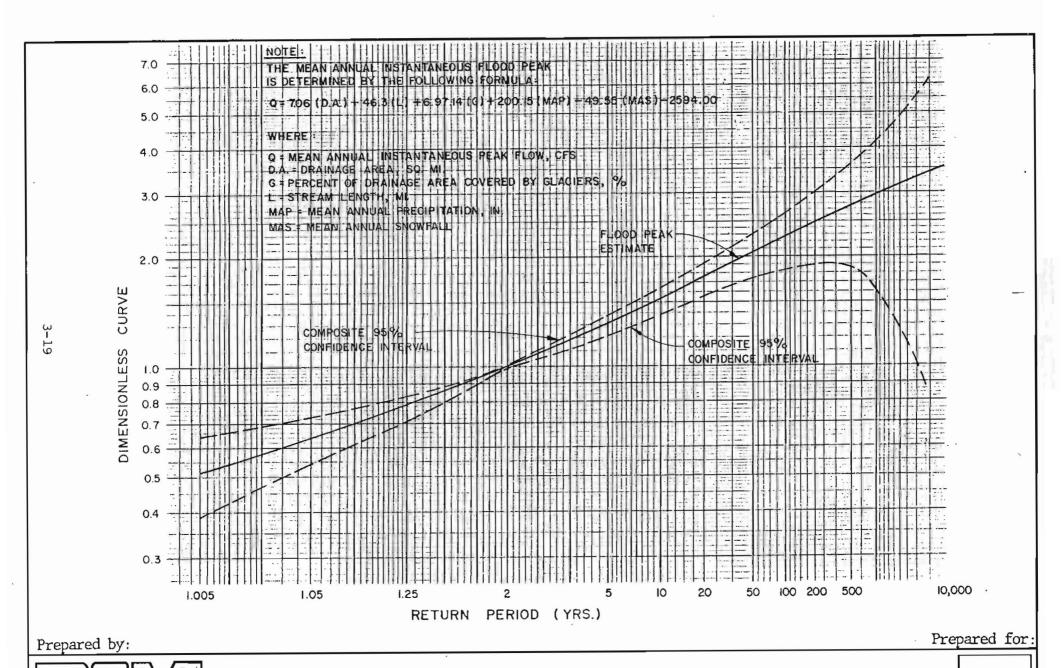
G = percent of basin as glaciers

MAP = mean annual precipitation, inches MAS = mean annual snowfall, inches

LP = percent of area as lakes and ponds

F = percent of area forested

T = mean minimum January temperature, °F

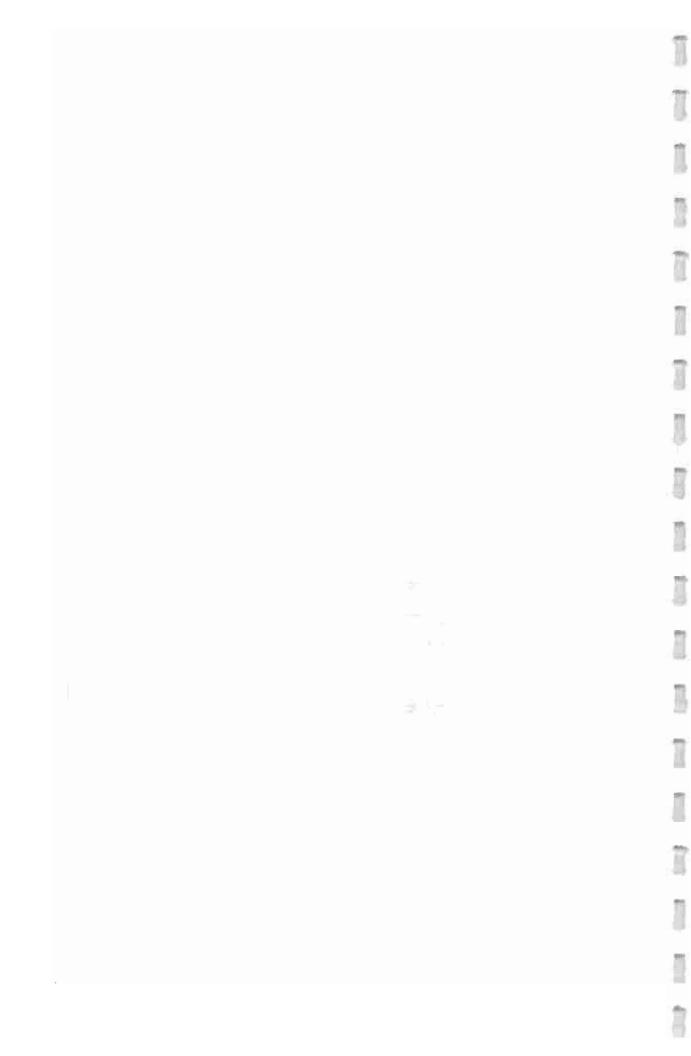


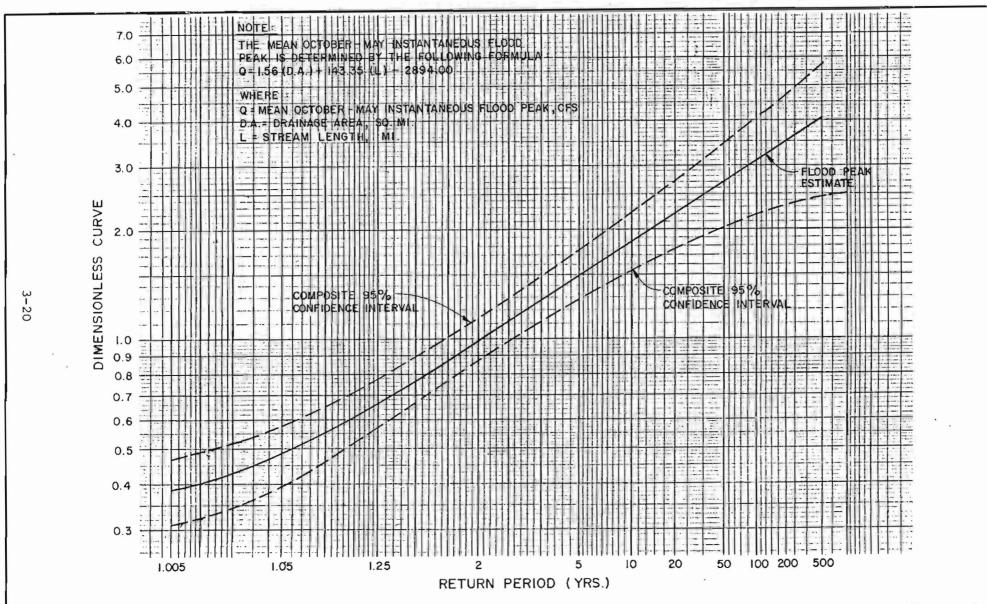
DESIGN DIMENSIONLESS REGIONAL FREQUENCY CURVE

FIGURE 3.1

ANNUAL INSTANTANEOUS FLOOD PEAKS

R&M CONSULTANTS, INC.





Prepared by:

Prepared for:



DESIGN DIMENSIONLESS REGIONAL FREQUENCY CURVE OCTOBER - MAY INSTANTANEOUS FLOOD PEAKS





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4 - EVALUATION OF DESIGN FLOODS

Using Figures 3.1 and 3.2, annual instantaneous flood peaks and October-May instantaneous flood peaks can be predicted with a minimum of additional data. The figures will prove useful in assessing flood peaks at ungauged sites within the study area. The figures should not be used for basins with drainage areas smaller that 164 square miles, the smallest basin area included in the study. Use of the equations and figures for these smaller basins may result in unrealistic values. It is recommended that the regression equations developed by Freethey and Scully (1980) be used for determining flood peaks in these basins. Their analysis included basins as small as one square mile, and the equations have a smaller standard error than the regressiona equations developed by Lamke (1979), as seen in Table 3.9.

It is necessary to have information concerning the drainage area, the length of the main channel, the percent of the drainage area covered by glaciers, the mean annual precipitation, and the mean annual snowfall in order to predict the annual instantaneous flood peaks on drainage basins in the upper Susitna River region. It is also necessary to have information on drainage area and stream length to predict October-May instantaneous flood peaks in the upper Susitna region. When compiling this information, it is important that the information come from the same source, or at least represent the same precision, as the data used in the regression analyses.

Thus, the drainage area should be computed from U.S. Geological Survey topographic maps in the horizontal plane. Main channel slope is taken as the average slope between points 10% and 85% of the distance along the main stream from the lowest point of interest to the basin divide. The percent of glacial area is the total drainage area shown as glaciers on the topographic maps divided by the basin drainage area, as measured by the grid-sampling method or planimeters. The mean annual precipitation and mean annual snowfall are determined from isohyetal maps (NWS, 1972 or Lamke, 1979) using the grid sampling method. An example of the use of the figures for determining instantaneous flood peaks is given below.

Suppose it is desired to obtain the annual instantaneous flood peak with a 50-year recurrence interval on an ungaged basin in the upper Susitna River basin. The following drainage basin characteristics were determined.

Area of Drainage Basin = 164 sq. mi. Stream Length = 25.0 miles Percent Area of Glaciers = 0 percent Mean Annual Precipitation = 40 inches Mean Annual Snowfall = 90 inches Using Equation (3.5), the mean annual instantaneous flood peak is determined to be 3269 cfs. Using Figure 3.1, the ratio of the 50-year peak to the mean annual peak is 2.04. Thus, the fifty year peak is

The confidence limits on this estimate could be found in the same manner by selecting the appropriate ratio from Figure 3.1. Thus the upper and lower 95% confidence interval are:

Lower 95% Confidence Interval
$$Q = 1.01 \times 3269 = 3,300 \text{ cfs}$$

5 - EVALUATION OF TYPICAL FLOOD HYDROGRAPHS FOR SUSITNA RIVER AT GOLD CREEK

For the gaging station on the Susitna River at Gold Creek, typical flood hydrographs were developed from several measured hydrographs. The temporal distribution of annual maximum flood peaks was first determined, and is presented in Figure 5.1. The figure illustrates that the greatest frequency of floods (55%) occurs in June and the second highest frequency of floods (26%) occurs in August.

The five largest peak hydrographs were selected for two periods: May-July (rain-and-snowmelt floods), and August-October (rainfall floods). These were made dimensionless by dividing the flows by the peak daily mean discharge, centered about the peak with ±15 days and plotted on the same graph. The mean hydrograph is the arithmetic average of the five floods selected for each period. The five dimensionless hydrographs and the corresponding mean curve for each time period are depicted in Figures 5.2 and 5.3. These figures illustrate that the mean curves are typical of the general shape of the snowmelt and rainfall hydrographs, but that there is considerable variation from year to year.

The seasonal discharge frequency curves depicted in Figure 5.4 were derived by assuming that all floods from May through July are rain and snowmelt floods and that all floods from August through October are rainfall floods with high-altitude snowmelt. The seasonal daily peak discharges for each period are tabulated in Table 5.1. The corresponding published annual instantaneous peaks are listed in the respective seasonal period. The average ratios between instantaneous peaks (Q_1) and daily peaks (Q_D) for the snowmelt and rainfall floods were calculated by averaging all the ratios (Q_1/Q_D) for corresponding instantaneous and daily peaks. The calculated ratios (Q_1/Q_D) are 1.063 for snowmelt events and 1.052 for rainfall events. The missing instantaneous peaks were then computed by multiplying the daily peak by the appropriate (Q,/QD) ratio. Using the three parameter log-normal frequency distribution and the instantaneous peaks from Table 5.1, the frequency curves for the May-July and August-October periods were computed. The annual instantaneous peak frequency curve is included on Figure 5.4 for comparison.

Fifty peak hydrographs were separated into the May-July and August-October periods and plotted. The base flows were estimated using standard techniques and subtracted from the hydrographs. The volume of each flood was estimated by planimetering the resulting hydrograph and tabulated in Table 5.2, together with the corresponding instantaneous peak. The flood volume frequency analysis for each period was conducted using the three parameter log-normal frequency distribution. The corresponding frequency curves with the 95% confidence levels are illustrated in Figures 5.5 and 5.6.

A power curve regression analysis was used to determine the correlation between the instantaneous peaks and the corresponding volumes. The correlation proved to be significant, with correlation coefficients of 0.794 and 0.825 for the May-July period and August-October period, respectively. The statistics are tabulated in Table 5.2 and illustrated graphically on Figures 5.7 and 5.8. As the correlation is acceptable, it allows for the development of the typical 100-, 500-, and 10,000-year flood hydrographs illustrated in Figures 5.9 and 5.10. These typical flood hydrographs were produced by using the general shape of the mean dimensionless hydrographs in Figures 5.2 and 5.3, and extrapolating the peaks and volumes for the 500- and 10,000-year floods from Figures 5.4, 5.5, and 5.6. The volumes beneath the flood hydrographs are those determined in the frequency analysis.

The typical hydrographs discussed here are qualitative indicators of flood hydrograph shapes. They should not be used for rigorous analytical calculations, but are intended to supplement them.

TABLE 5.1
DETERMINATION OF DAILY INSTANTANEOUS PEAK RATIO
SUSITNA RIVER AT GOLD CREEK

A MED DOOR TA TORKE AND A I

	May - July (Snowmelt Floods)		August - October (Rainfall Floods)			
	Daily Peaks	Instant- aneous Peaks	Ratio	Daily Peaks	Instant- aneous Peaks	Ratio
Year	(cfs)	(cfs)	Q _I /Q _D	(cfs)	(cfs)	Q _I /Q _D
49	J1 -	000150	4,13	35,000	[38,700]	
50	34,000	(37,300)	1.097	27,600	[29,000]	
51	35,800	(37,400)	1.044	31,800	[33,500]	
52	43,300	(44,700)	1.032	41,900	[44,100]	
53	37,700	(38,400)	1.065	28,100	[29,600]	
54	30,100	[32,000]		41,000	(42,400)	1.034
55	39,000	[41,500]		56,900	(58,100)	1.021
56	51,500	(51,700)	1.004	31,000	[32,600]	
57	40,600	(42,200)	1.039	26,600	[28,000]	
58	28,000	[29,800]		47,800	(49,600)	1.038
59	39,600	[42,100]		59,700	(62,300)	1.044
60	39,300	[41,800]		40,100	(41,900)	1.045
61	54,000	(59,300)	1.098	26,000	[27,400]	
62	79,700	(80,600)	1.011	31,000	[32,600]	
63	49,000	(53,800)	1.098	35,000	[36,800]	
64	85,900	(90,700)	1.056	21,600	[22,800]	
65	39,900	(43,600)	1.093	33,600	[35,400]	
66	58,400	(63,600)	1.089	33,500	[35,200]	
67	50,000	[53,150]	F.11.	76,000	(80,200)	1.055
68	39,700	(41,800)	1.053	21,800	[22,900]	
69	26,500	(28,400)	1.072	16,800	[17,700]	
70	30,800	[32,740]		31,600	(33,400)	1.057
71	66,300	[70,500]		77,700	(87,400)	1.125
72	70,700	(82,600)	1.168	26,400	[27,800]	
73	52,800	(54,100)	1.025	30,500	[32,100]	
74	33,600	(37,200)	1.107	22,300	[23,500]	
75	44,000	(47,300)	1.075	24,800	[26,100]	
76	33,300	(35,700)	1.072	32,000	[33,700]	
77	52,600	(54,300)	1.032	26,200	[27,600]	
78	24,300	(25,000)	1.029	20,800	[21,900]	
79	39,300	(41,300)	1.051	28,400	[29,900]	
80	49,700	(52,000)	1.046	31,100	[32,800]	
			$Q_1/Q_D = X = 1.063$			$Q_1/Q_D = X = 1.052$

S.D.=.034

N = 23

S.D.=.032

N=8

Notes: () Published instantaneous peaks

^[] Instantaneous peaks computed by ratio.

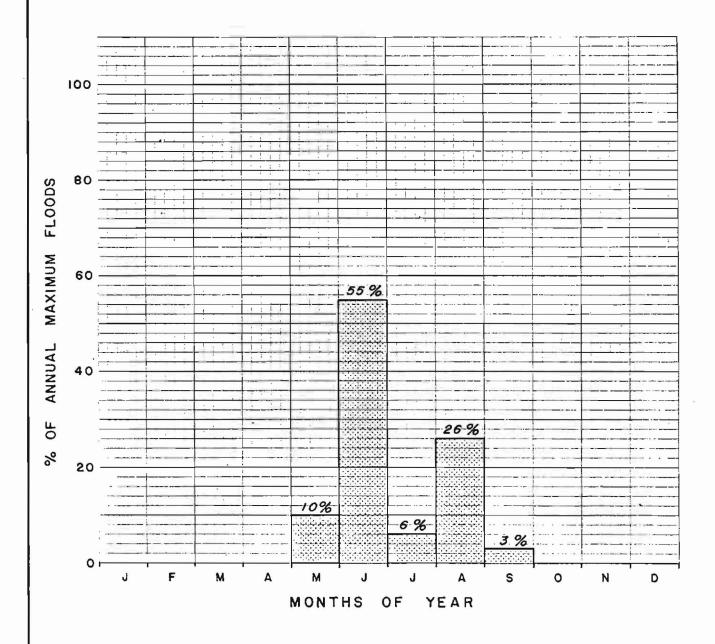
TABLE 5.2
TABULATION AND STATISTICS OF INSTANTANEOUS PEAKS
VERSUS PEAK VOLUMES
SUSITNA RIVER AT GOLD CREEK

		May - July		August - October (Rainfall Floods)	
	(Snowmelt				
	Instantaneous	Peak Volume, V	Instantaneous Peak, Q _p	Peak Volume, V	
Year	Peak, Qp				
1 Cat	(cfs)	$(ft^3 \times 10^9)$	(cfs)	$(\mathrm{ft}^3 \times 10^9)$	
50	37,300	10.6	29,000	8.2	
51	37,400	23.0	33,500	16.4	
52	44,700	69.9	44,100	12.9	
53	38,400	33.6	29,600	8.4	
54	32,000	9.7	42,400	ND 10.0	
55 56	41,500	38.0	58,100	19.0 ND	
57	51,700 42,200	33.8 27.0	32,600 28,000	6.1	
58	29,800	ND	49,600	19.0	
59	42,100	28.4	62,300	37.6	
60	41,800	18.4	41,900	10.3	
61	59,300	30.8	27,400	ND	
62	80,600	63.7	32,600	ND	
63	53,800	26.7	36,800	5.7	
64	90,700	83.4	22,800	6.5	
65	43,600	35.6	35,400	17.7	
66	63,600	30.8	35,200	12.5	
67	53,150	19.9	80,200	30.3	
68	41,800	32.2	22,900	UPH	
69	28,400	11.8	17,700	3.9	
70	32,740	16.1	33,400	7.7	
71	70,500	50.1	87,400	27.5 UPH	
72	82,600	82.8 36.8	27,800 32,100	14.9	
73 74	54,100 37,200	23.9	23,500	9.3	
74 75	47,300	28.0	26,100	UPH	
76	35,700	18.8	33,700	11.5	
77	54,300	45.9	27,600	3.2	
78	25,000	12.9	21,900	UPH	
79	41,300	12.6	29,900	UPH	
80	52,000	NA	31,100	NA	
STATISTICS					
Mean	48,441	32.9×10^9	40,090	13.7 × 10 ⁹	
	•		18,214	9.0 × 10 ⁹	
Standard Deviation	16,104	20.1 × 10 ⁹ 1.50	10,214	1.33	
Regression Equatio	n V = 2		V = 9		
Correlation Coeffic	ient r =	r = 0.794 $r = 0.825$		= 0.825	
Sample Size	n	= 29	n	= 21	

Notes: ND = No data

UPH = Undefined peak hydrograph

NA = Not available



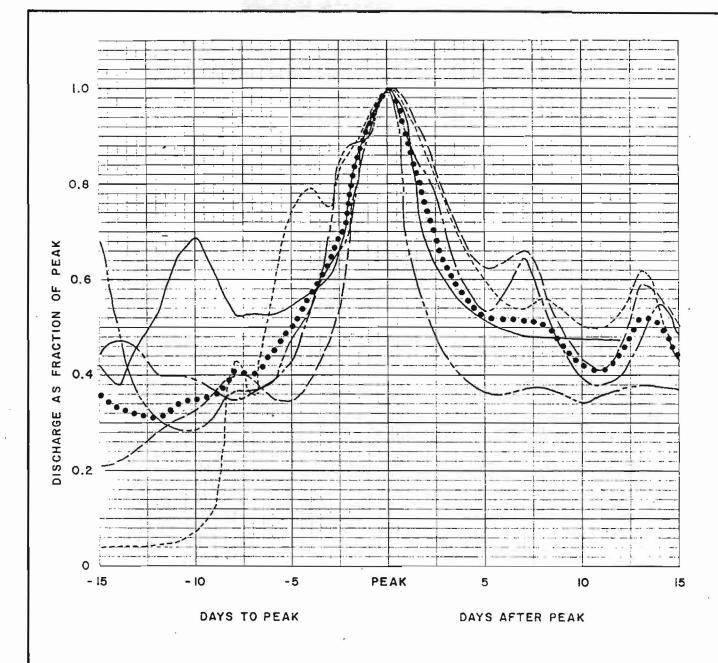
SUSITNA RIVER AT GOLD CREEK PERIOD OF RECORD-1950-1980

Prepared by:

Prepared for



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SUSITNA RIVER AT GOLD CREEK

LEGEND

	Date	Peak Discharge (cfs)	
•••••	Mean Curve		
	June 23,1961	54,000	
	June 15, 1962	79,900	
	June 7, 1964	85,900	
	June 6, 1966	58,400	
	June 17, 1972	70,700	

Prepared by:

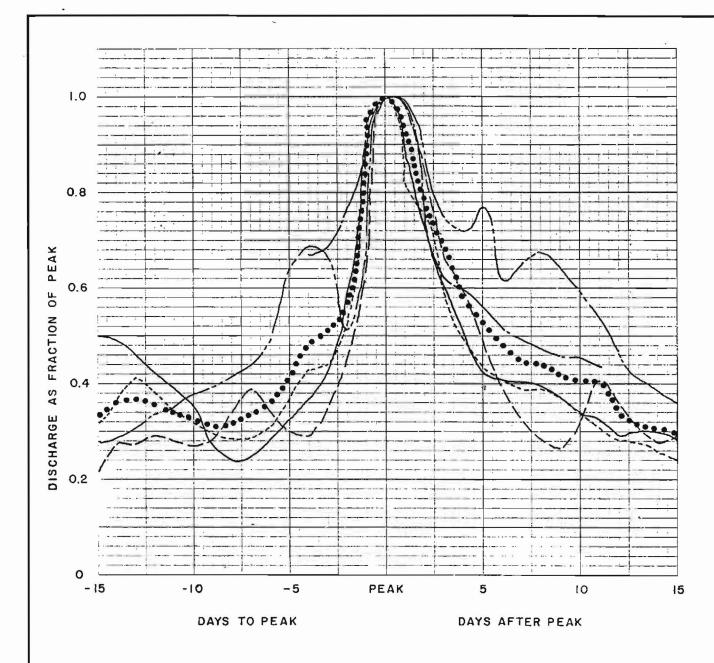
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DIMENSIONLESS HYDROGRAPHS
MAY - JULY
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SUSITNA RIVER AT GOLD CREEK

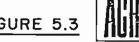
LEGEND

		Date	Peak Discharge (cfs)
• • • • • •	Mean	Curve	
	Aug	26,1955	56,900
	Aug	3,1958	47,800
	Aug	24,1959	59,700
		15,1967	
	Aug	10, 1971	77,700

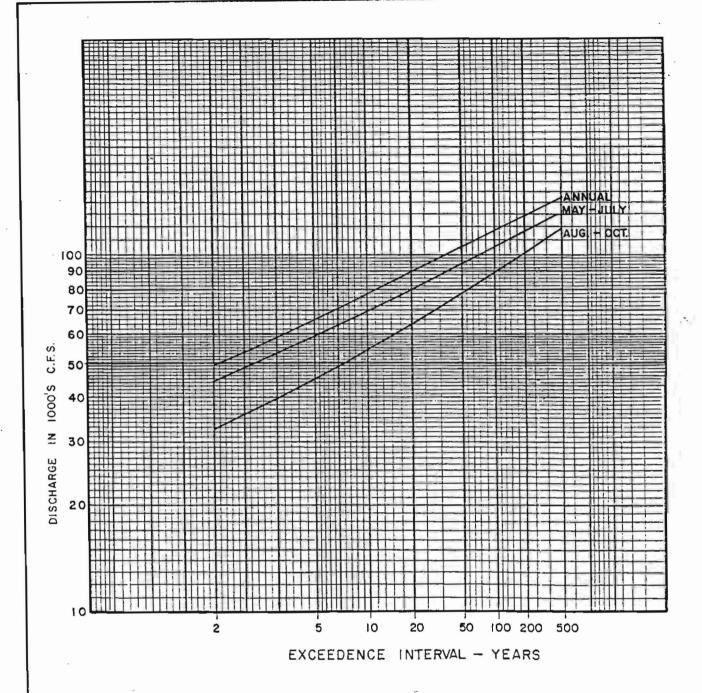
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DIMENSIONLESS HYDROGRAPHS AUG - OCT



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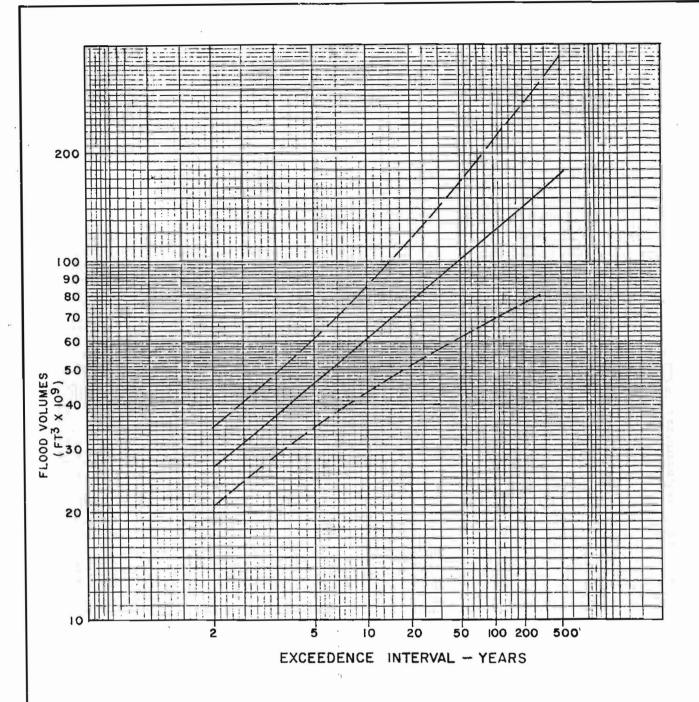
SUSITNA RIVER AT GOLD CREEK
PERIOD OF RECORD -1950-1980
ANNUAL SKEW IS 0.6830
MAY-JULY SKEW IS 1.130
AUG-OCT SKEW IS 1.134

Prepared by:

Prepared for







SUSITNA RIVER AT GOLD CREEK
PERIOD OF RECORD - 29 yrs.

LEGEND

---- 95% CONFIDENCE LIMITS
---- VOLUME FREQUENCY CURVE

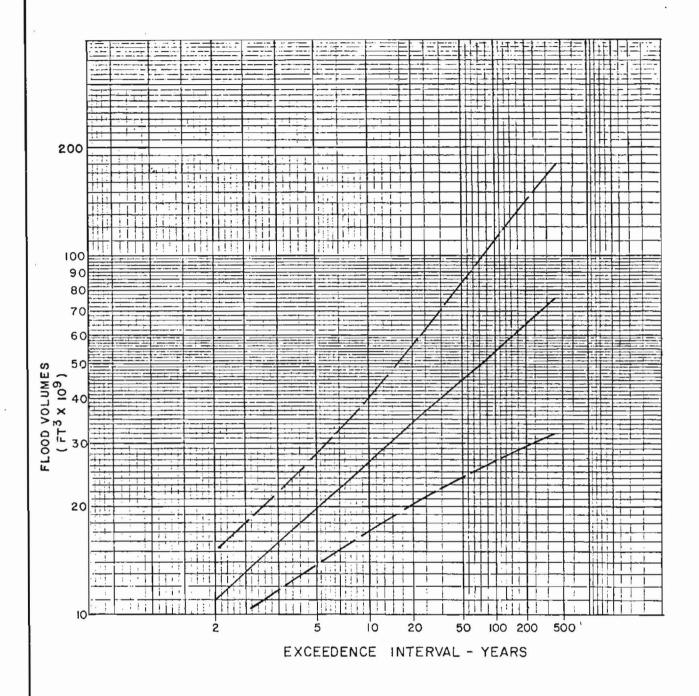
Prepared by:

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FLOOD VOLUME FREQUENCY CURVE
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SUSITNA RIVER AT GOLD CREEK
PERIOD OF RECORD - 21 yrs.

LEGEND

--- 95 % CONFIDENCE LIMITS
--- VOLUME FREQUENCY CURVE

Prepared by:

Prepared for

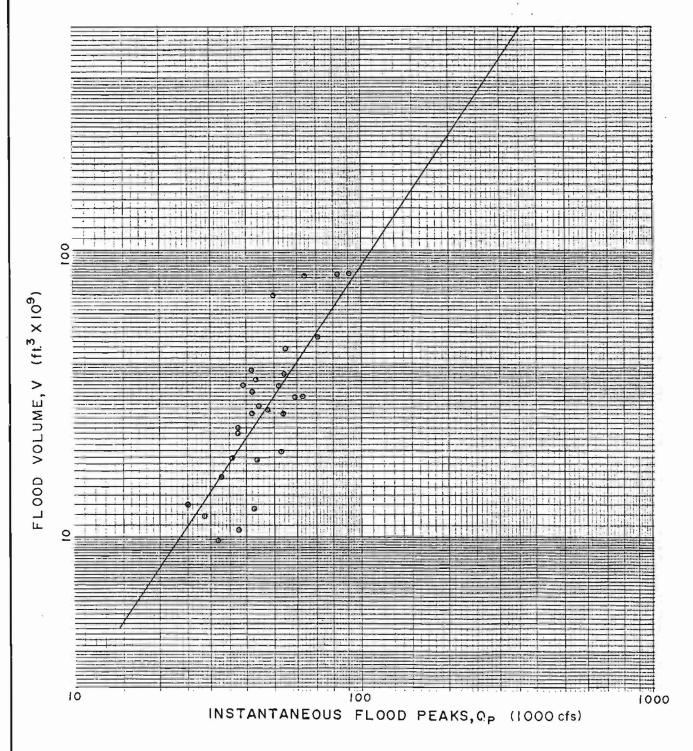


FLOOD VOLUME FREQUENCY CURVE

AUG-OCT

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SUSITNA RIVER AT GOLD CREEK PERIOD OF RECORD - 29 YEARS

 $V = 2820 Q_{P}^{1.50}$

 $r^2 = 0.63$

n = 29

Prepared by:

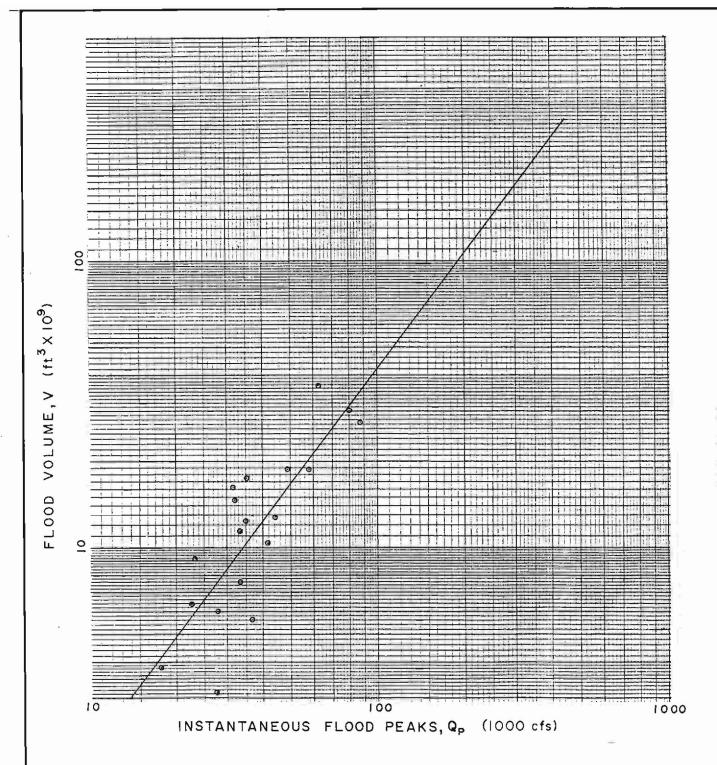
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VOLUME vs. PEAK
MAY - JULY

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SUSITNA RIVER AT GOLD CREEK PERIOD OF RECORD - 29 YEARS.

 $V = 9840 Q^{1.33}$

 $r^2 = 0.68$

n = 21

Prepared by:

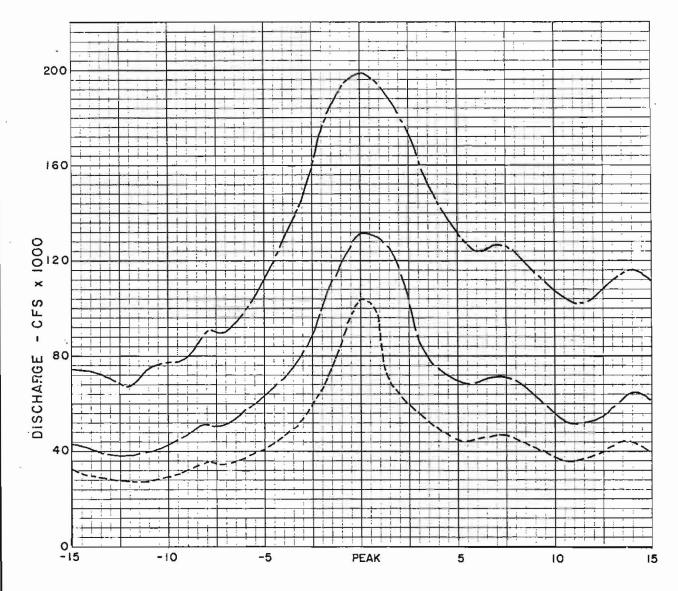
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VOLUME VS. PEAK AUG. — OCT. 5-12



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TIME - DAYS

SUSITNA RIVER AT GOLD CREEK
100,500,10000 yr. FLOOD VOLUMES

LEGEND

	Flood Volume	Peak Discharge
		<u>(cfs)</u>
100 yr	122.3 X 10 ⁹	104,550
— — 500 yr	178.2 X 10 ⁹	131,870
l0,000 y	310.0 X 10 9	198,000

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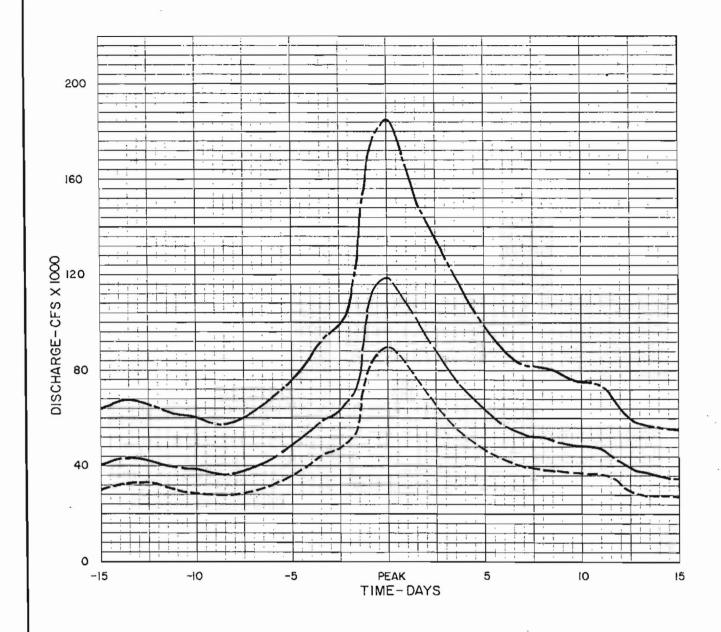


FLOOD HYDROGRAPHS

MAY - JULY

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SUSITNA RIVER AT GOLD CREEK

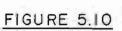
LEGEND

	Flood Volume ft ³	Peak Discharge (cfs)
100 yr	53.8 X 109	90,140
—— 500 yr	78.8 X 10 ⁹	119,430
10,000 yr	140.0 X 10 9	185,000

PREPARED FOR



FLOOD HYDROGRAPHS AUG - OCT



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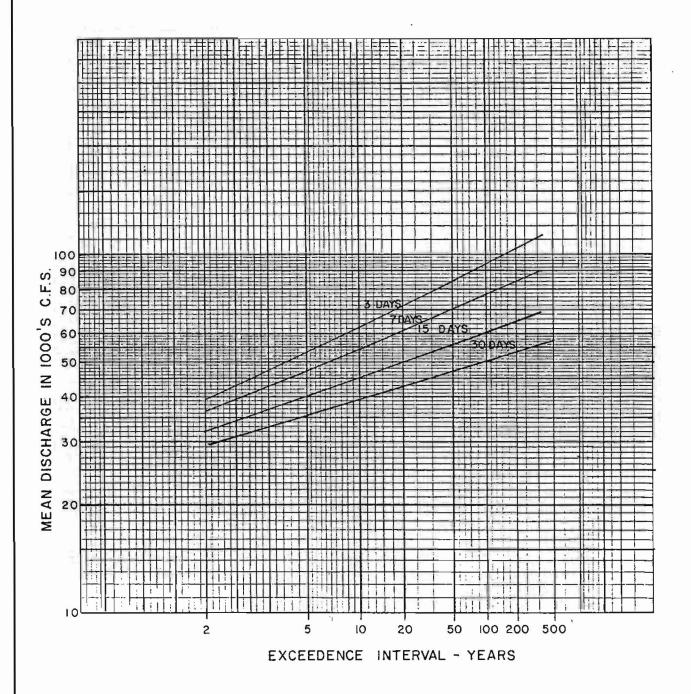
6 - FLOOD VOLUME-DURATION FREQUENCY ANALYSIS

To assist in designing temporary diversion structures and determining reservoir filling sequences, a flood volume-duration frequency analysis was conducted for May-July flows of the Susitna River at Gold Creek. Table 6.1 is a computer tabulation of May through July volume durations computed by the U.S. Geological Survey. The mean discharge for the 3-, 7-, 15- and 30-day durations were utilized with the three parameter log-normal frequency distribution to compute the volume-duration frequency curves illustrated on Figure 6.1.

SUSITNA RIVER AT GOLD CREEK

HIGHEST MEAN DISCHARGE (CFS) OF CONSECUTIVE DAYS FROM MAY THROUGH JULY VOLUME DURATIONS COMPUTED BY U.S. GEOLOGICAL SURVEY

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YEAR	* 1	3	. 7	15	30 22700.0 27
1950	34000.0 23	32100.0 24	27000.0 28	24500.0 28	22100.0 21
1951	35800.0 22	32100.0 25	29800.0 24	25200.0 27	22700.0 28
1952	43300.0 13	42300.0 12	39500.0 11	37000.0 9	33000.0 10
1953	37700.0 21	37200.0 18	35500.0 17	31000.0 19	27400.0 19
1954	30100.0 28	29500.0 28	28800.0 25	26900.0 24	25700.0 22
1955	39000.0 20	38700.0 16	36600.0 16	35500 • 0 12	33800.0 B
	51500.0 9	49900.0 8	46800.0 7	40300+0 4	34500.0 7
1956		39900.0 14	38400.0 13	35500.0 13	32300.0 13
1957	40600.0 14 32400.0 26	32100.0 26	28000.0 26	28 0 • 0 0 0 82	25800.0 21
1958		37200.0 19	30700.0 22	27300.0 23	26200.0 20
1959	39600.0 18	36100.0 20	32900.0 19	26100.0 25	23100.0 26
1960	40000.0 15	30100 0 20	51.7.7.00	in the	
1041	54000.0 6	52000.0 7	42700.0 9	36800.0 10	30800.0 14
1961	79900.0 2	75200.0 ?	64700.0 2	53200.0 2	43300.0 2
1962	49000.0 11	463.00.0 9	42900.0 8	39200.0 7	34900.0 6
1963	85900.0 l	81900.0 1	75000.0 1	63500.0 1	50700.0 1
1964	39900.0 16	37400.0 17	34100.0 18	31300.0 18	30500.0 15
1965	39900.0 10	3/400.0 11			a.*
1044	58400.0 5	56600.0 5	49200.0 4	39800.0 5	33000.0 9
1966	50000.0 10	46300.0 10	38900.0 12	32500.0 16	29900.0 16
1967	39700.0 17	38900.0 15	38100.0 14	36500.0 11	32400.0 11
1968	26500.0 29	24400.0 29	- 20500.0 30	17700.0 30	16700.0 30
1969		30100.0 27	27100.0 27	25700.0 26	23800.0 24
1970	30800.0 27	30100 0 27			
1071	66300.0 4	59000.0 4	48300.0 5	38200.0 8	35400.0 5
1971	70700.0 3	65600.0 3	53500.0 / 3	39800.0 6	37300.0 4
1972	52800.0 7	45300.0 11	40900.0 10	34600.0 14	28600.0 18
1973	33600.0 24	33200.0 22	32400.0 21	29300.0 20	23200.0 25
1974	44000.0 12	42200.0 13	37100.0 15	33300.0 15	32300.0 12
1975	44000.0 12	7220010 13		,	
1976	33300.0 25	32100.0 23	29800.0 23	28600:0 21	24500.0 23
1976	52600.0 8	52200.0 6	48300.0 6	41500.0 3	38000.0 3
1977	24300.0 30	23800.0 30	23100.0 29	22600:0 29	21700.0 29
1978	39300.0 19	34800.0 21	32600.0 20	31700:0 17	29100.0 17
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SUSITNA RIVER AT GOLD CREEK
PERIOD OF RECORD-1950-1979

Prepared by:

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VOLUME DURATION FREQUENCY CURVES MAY - JULY

FIGURE 6.1

7 - REFERENCES

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ATTACHMENT A

FLOOD FREQUENCY CURVES FOR ANNUAL INSTANTANEOUS FLOOD PEAK SERIES, INDIVIDUAL STATIONS

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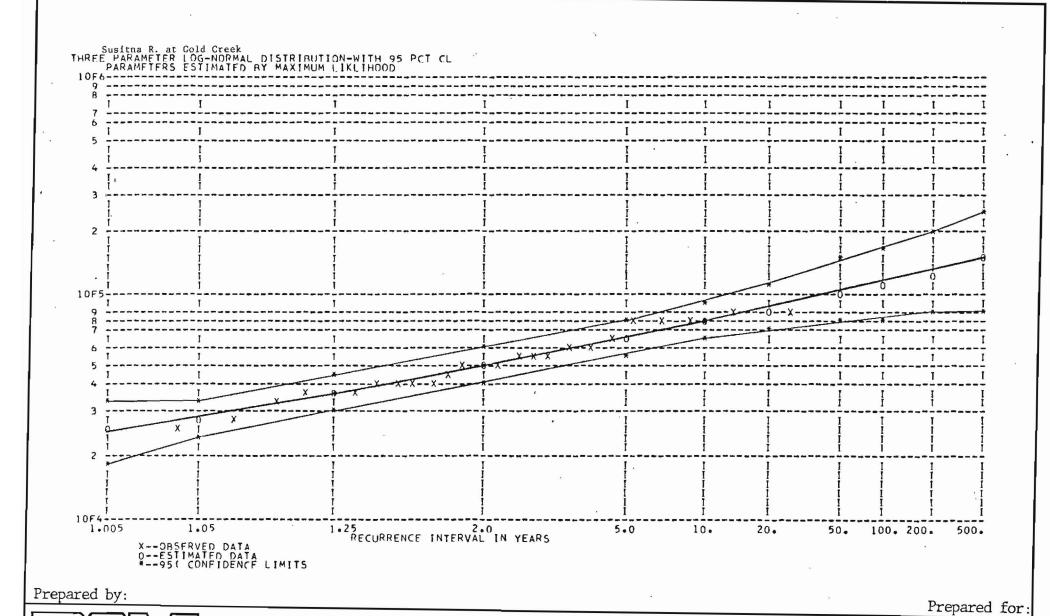
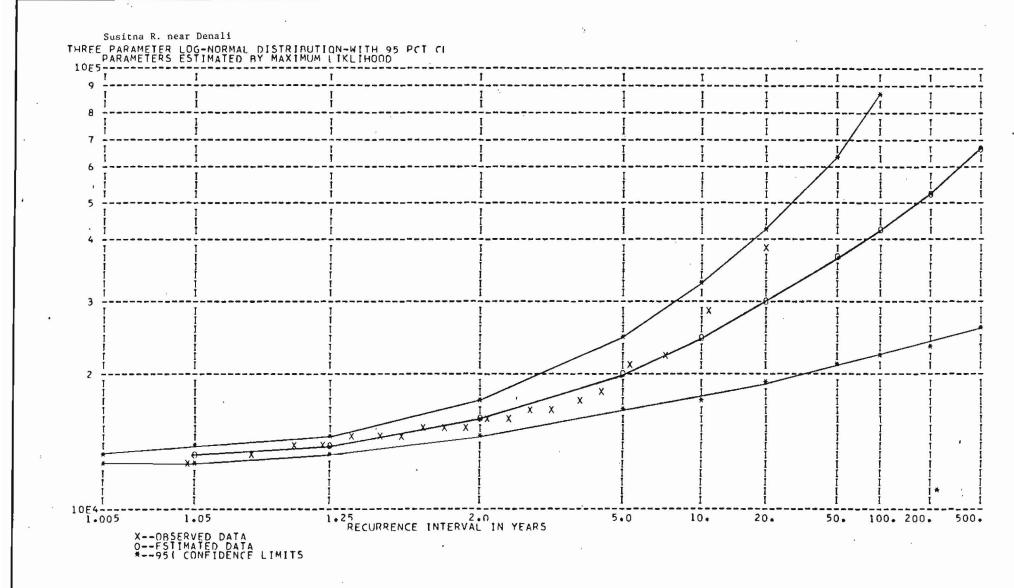


FIGURE: A.1

ANNUAL FLOOD FREQUENCY CURVE SUSITNA RIVER AT GOLD CREEK

REM CONSULTANTS, INC.





Prepared by:

Prepared for:



ANNUAL FLOOD FREQUENCY CURVE SUSITNA RIVER NEAR DENALI



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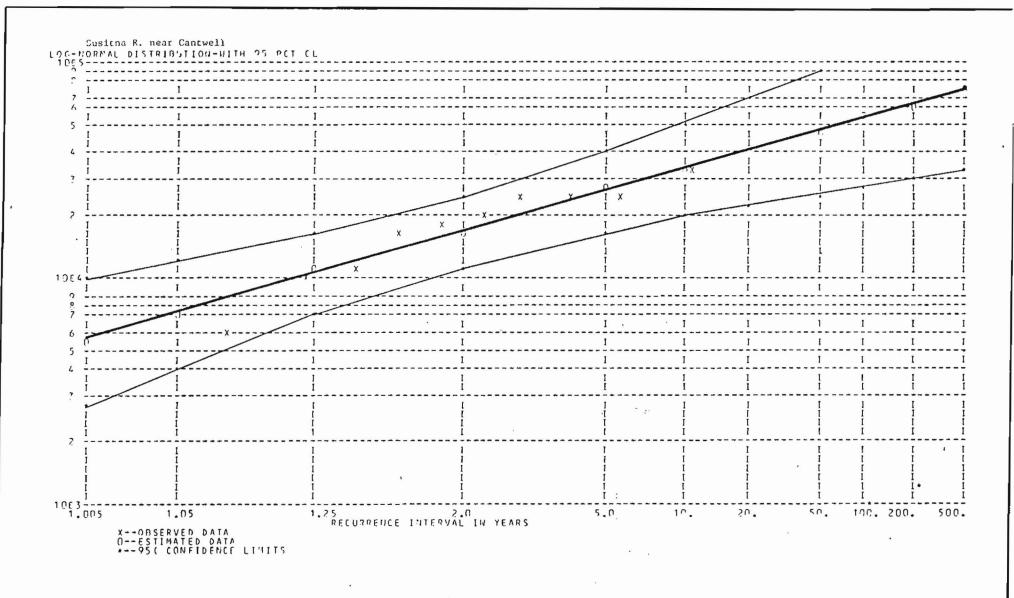


ANNUAL FLOOD FREQUENCY CURVE
MACLAREN RIVER NEAR PAXSON



FIGURE: A.3

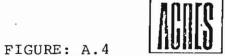
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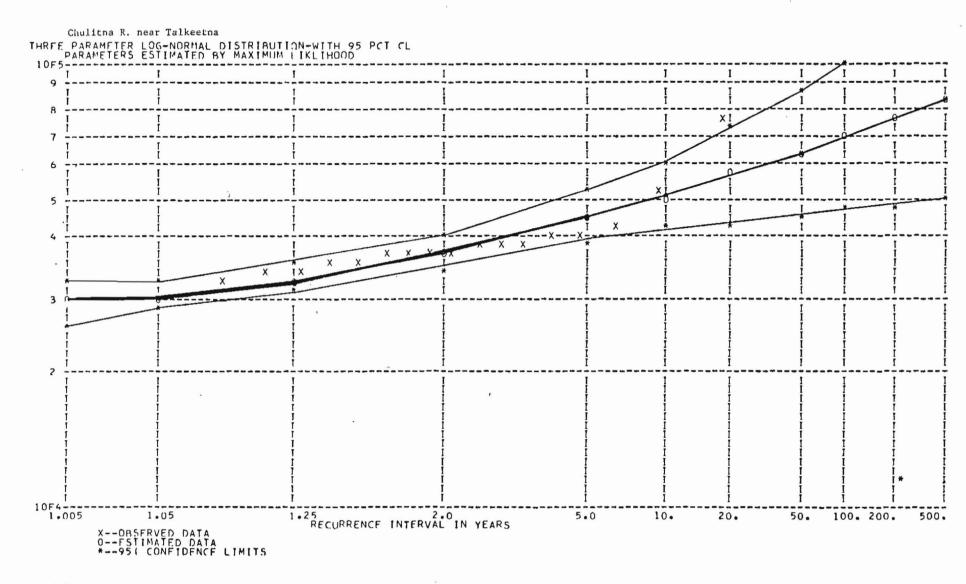
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ANNUAL FLOOD FREQUENCY CURVE SUSITNA RIVER NEAR CANTWELL







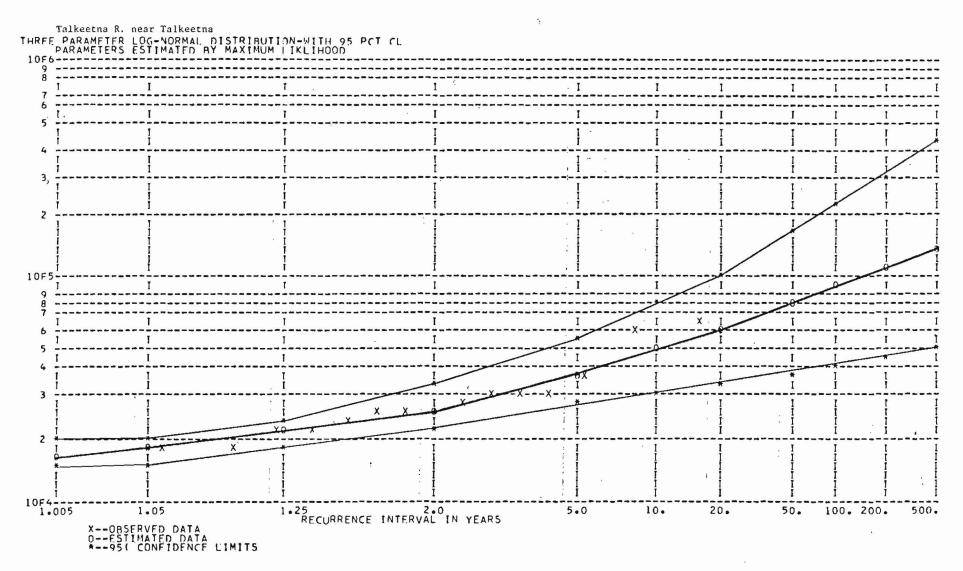
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ANNUAL FLOOD FREQUENCY CURVE CHULITNA RIVER NEAR TALKEETNA



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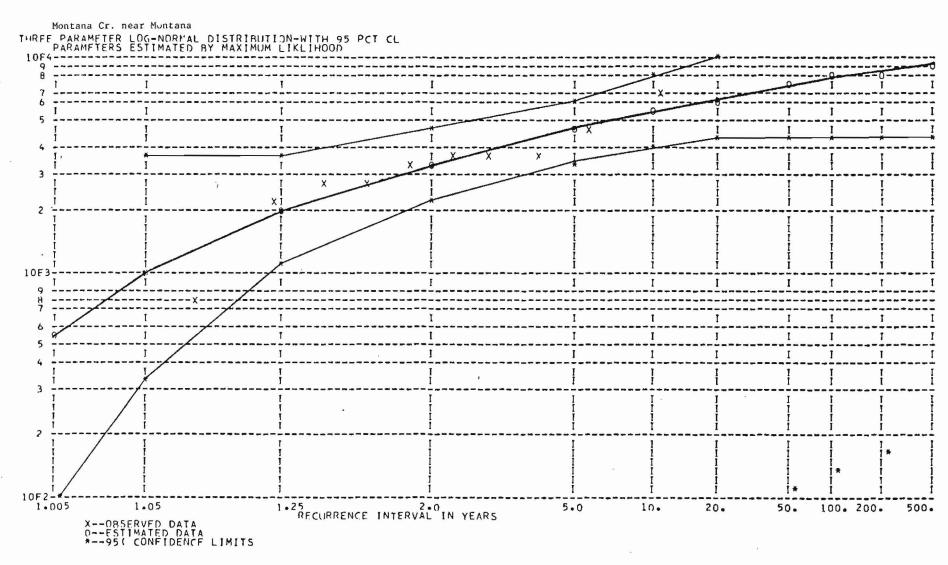
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ANNUAL FLOOD FREQUENCY CURVE
TALKEETNA RIVER NEAR TALKEETNA



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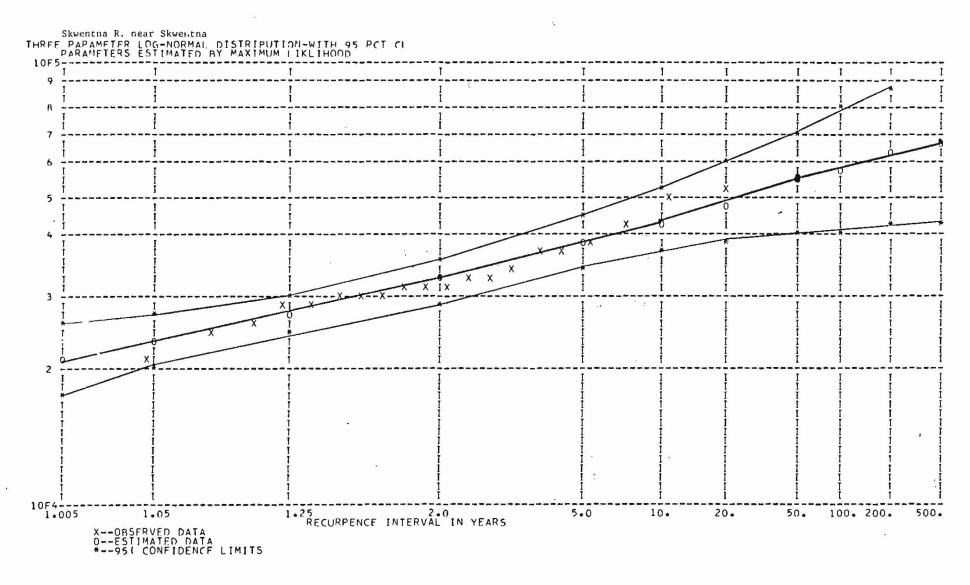


ANNUAL FLOOD FREQUENCY CURVE MONTANA CREEK NEAR MONTANA



FIGURE: A.7

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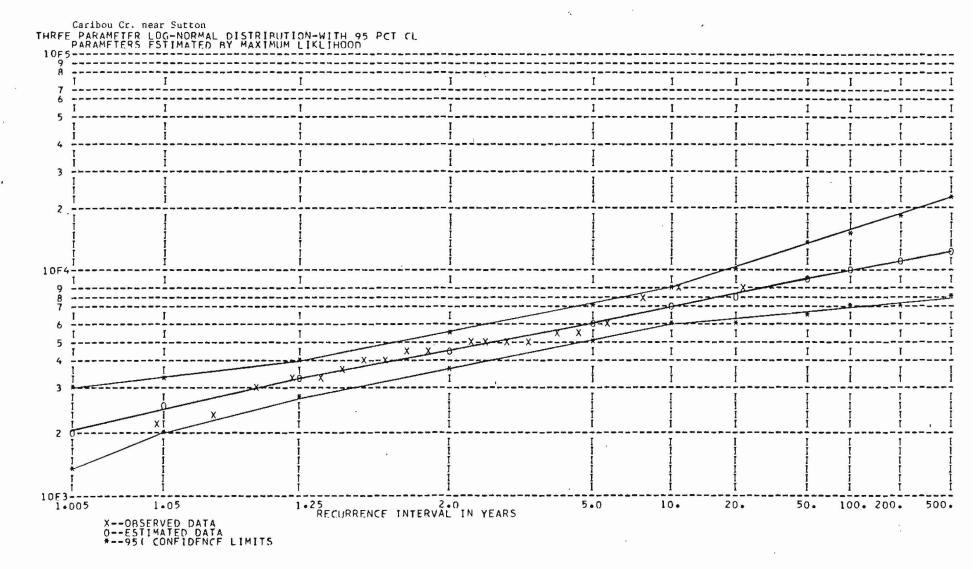
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ANNUAL FLOOD FREQUENCY CURVE SKWENTNA RIVER NEAR SKWENTNA



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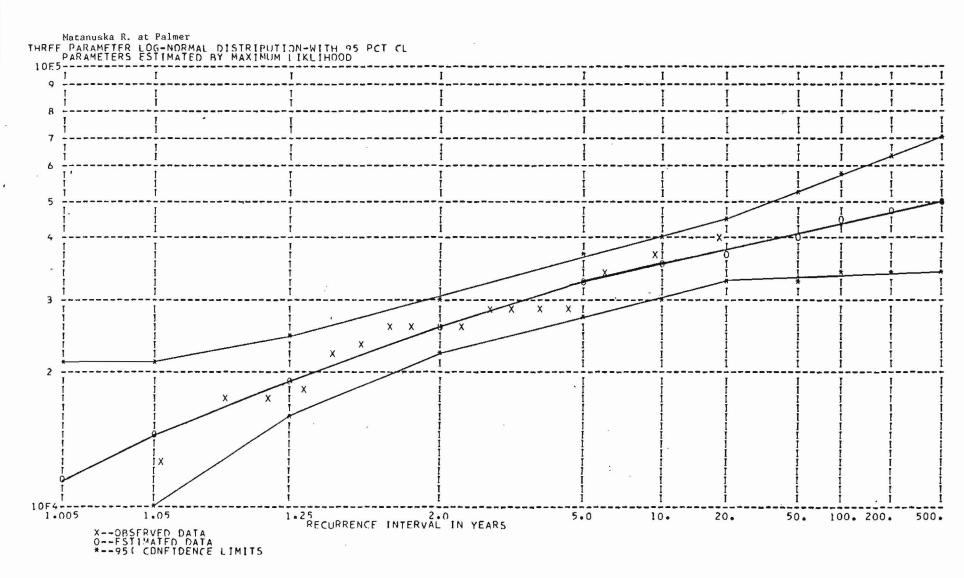
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ANNUAL FLOOD FREQUENCY CURVE CARIBOU CREEK NEAR SUTTON



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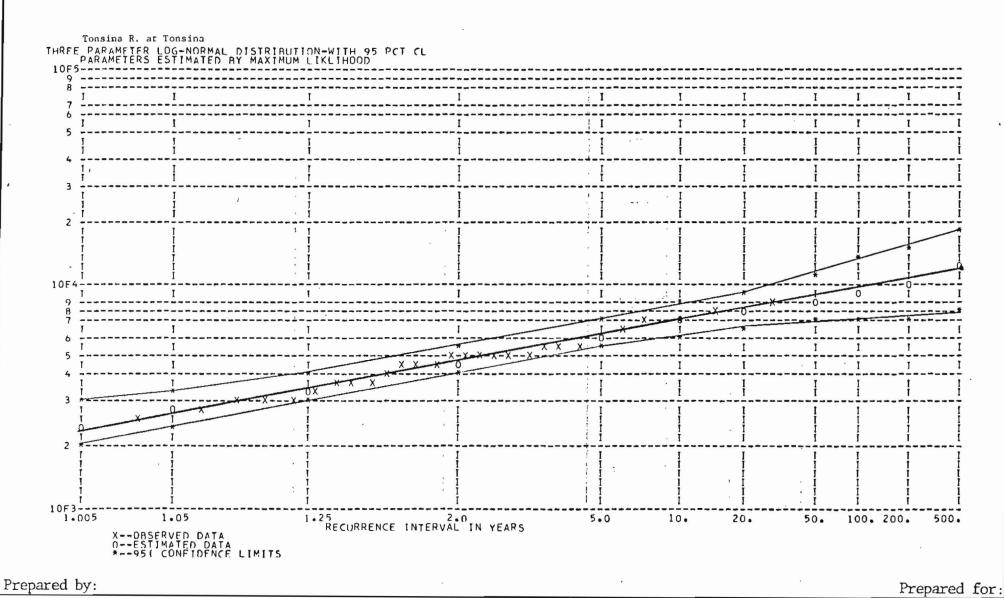
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ANNUAL FLOOD FREQUENCY CURVE MATANUSKA RIVER AT PALMER







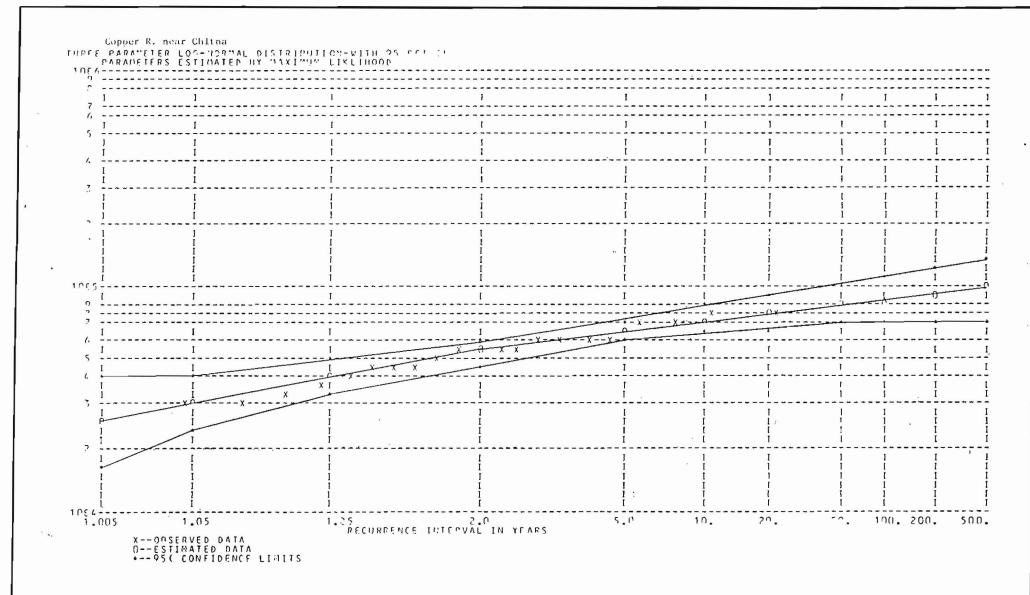
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ANNUAL FLOOD FREQUENCY CURVE TONSINA RIVER AT TONSINA



FIGURE: A.11

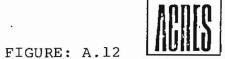




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ANNUAL FLOOD FREQUENCY CURVE COPPER RIVER NEAR CHITINA

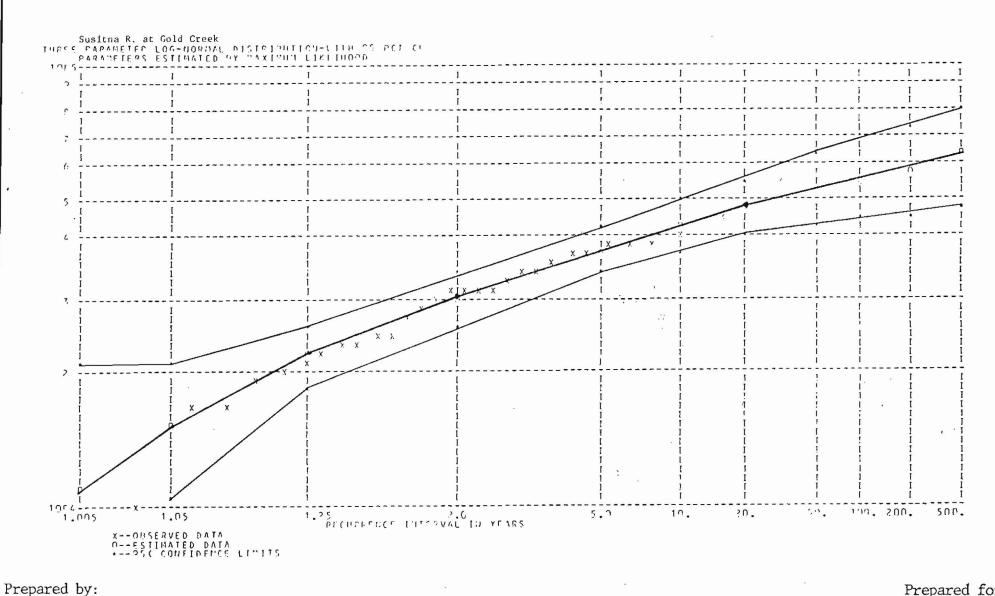


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ATTACHMENT B

FLOOD FREQUENCY CURVES FOR
MAXIMUM ANNUAL OCTOBER-MAY MEAN,
INDIVIDUAL STATION





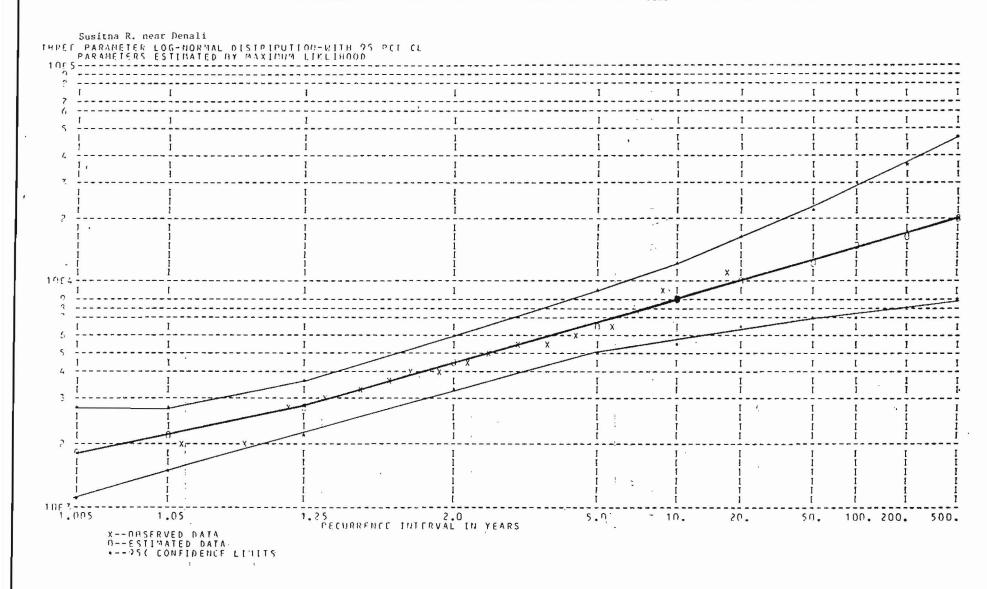
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OCT-MAY FLOOD FREQUENCY CURVE SUSITNA RIVER AT GOLD CREEK Prepared for:



FIGURE: B.1

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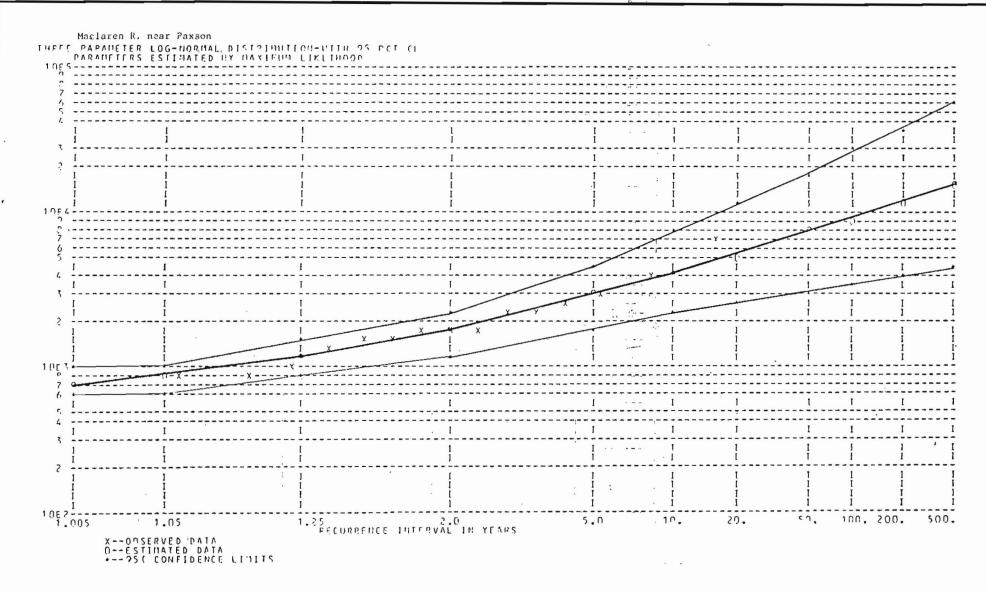


OCT-MAY FLOOD FREQUENCY CURVE SUSITNA RIVER NEAR DENALI

FIGURE: B.2







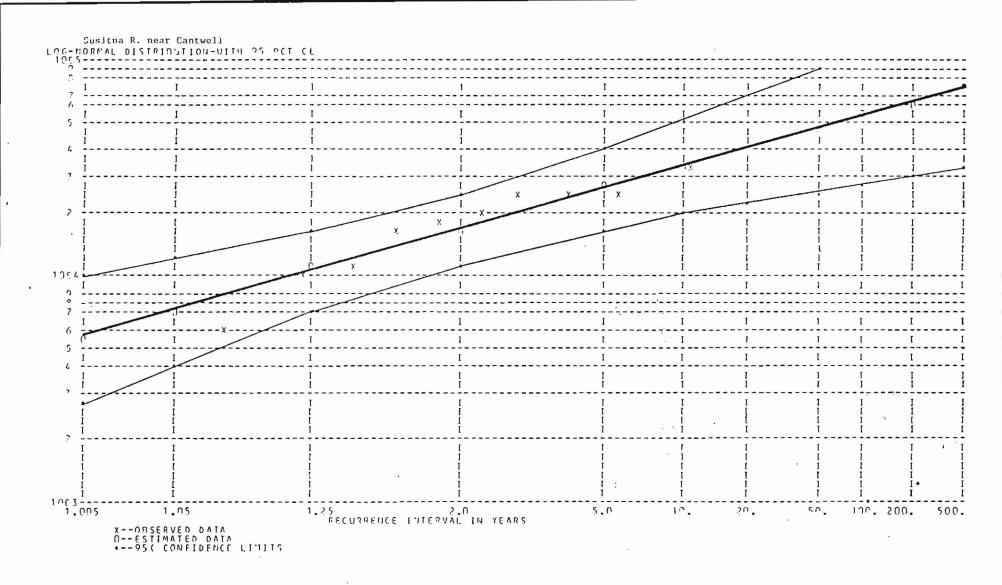
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OCT-MAY FLOOD FREQUENCY CURVE MACLAREN RIVER NEAR PAXSON



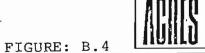
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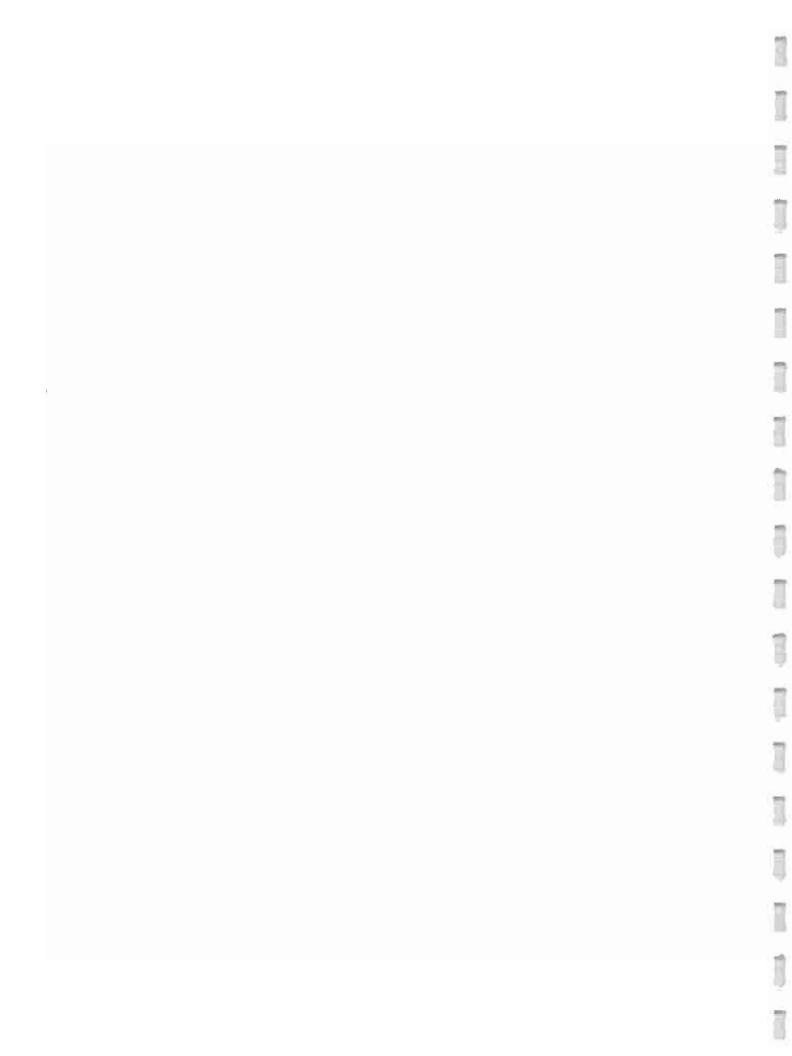


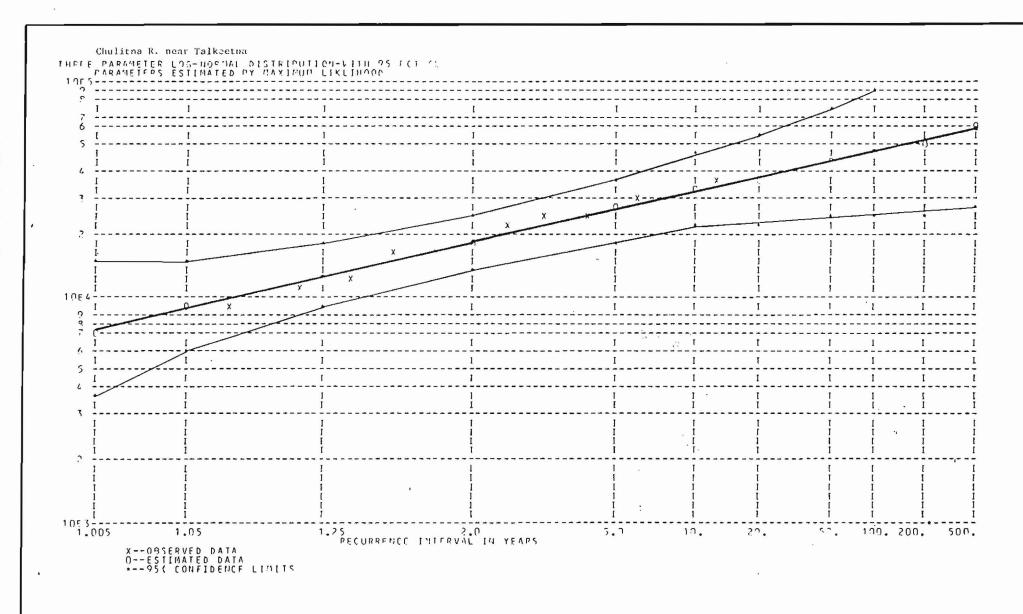
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OCT-MAY FLOOD FREQUENCY CURVE SUSITNA RIVER NEAR CANTWELL





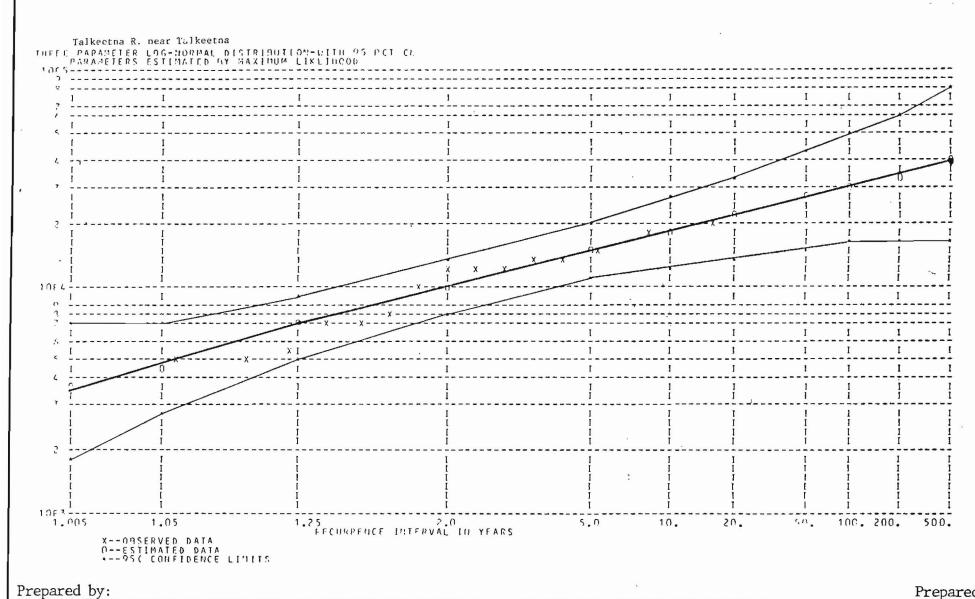


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OCT-MAY FLOOD FREQUENCY CURVE CHULITNA RIVER NEAR TALKEETNA



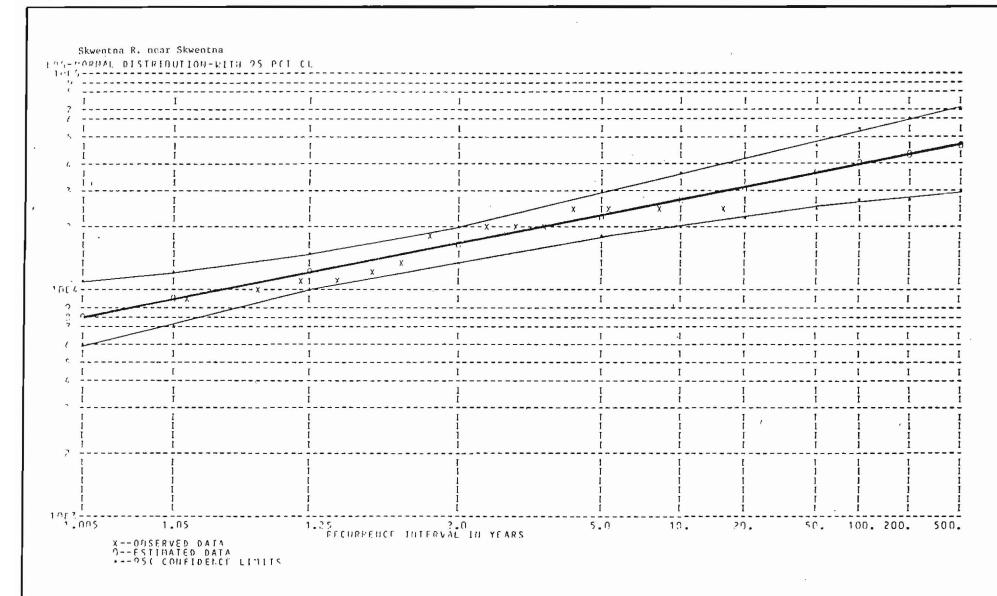


REM CONSULTANTS, INC.

OCT-MAY FLOOD FREQUENCY CURVE TALKEETNA RIVER NEAR TALKEETNA Prepared for:



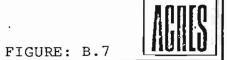
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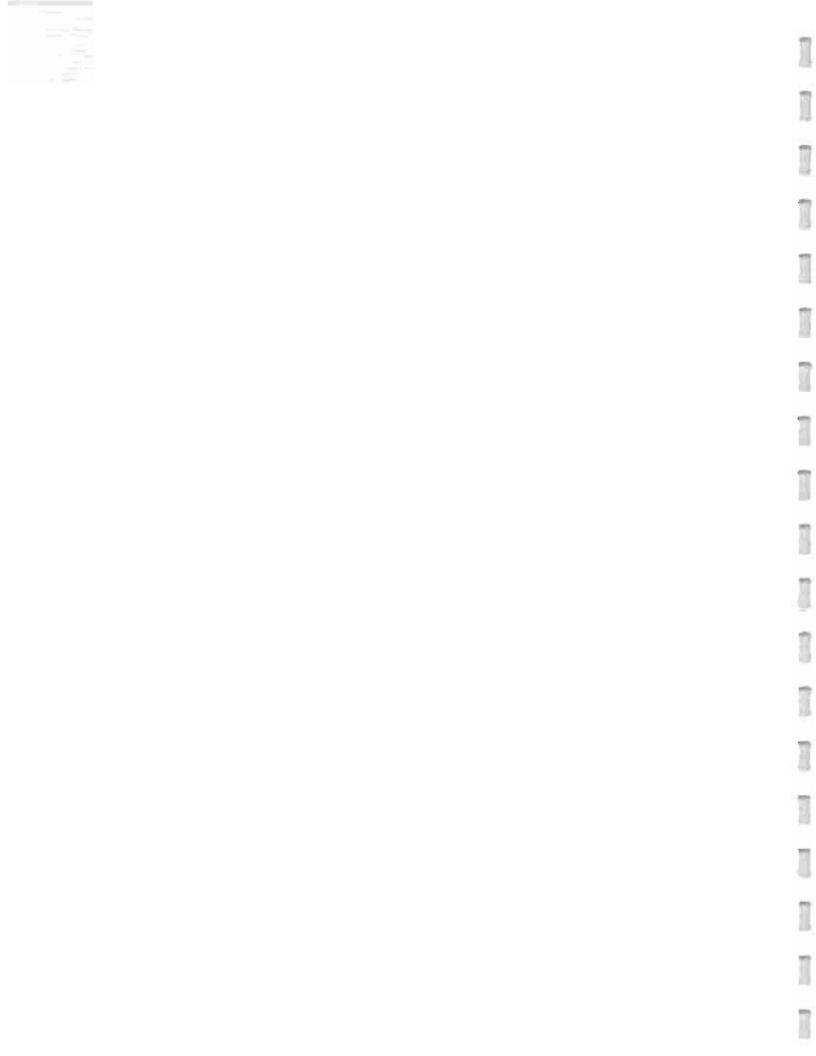


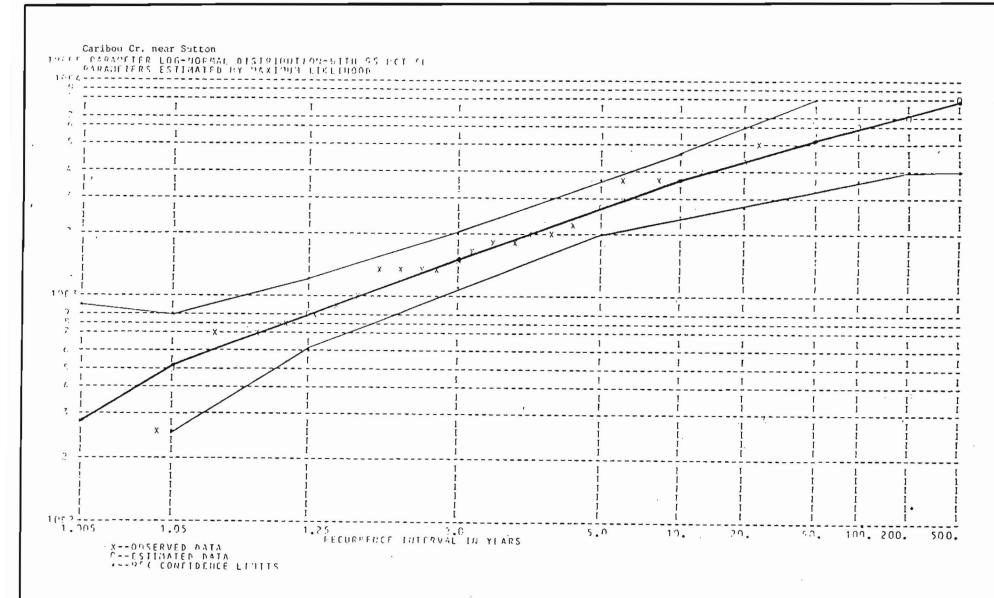
Prepared for:



OCT-MAY FLOOD FREQUENCY CURVE SKWENTNA RIVER NEAR SKWENTNA





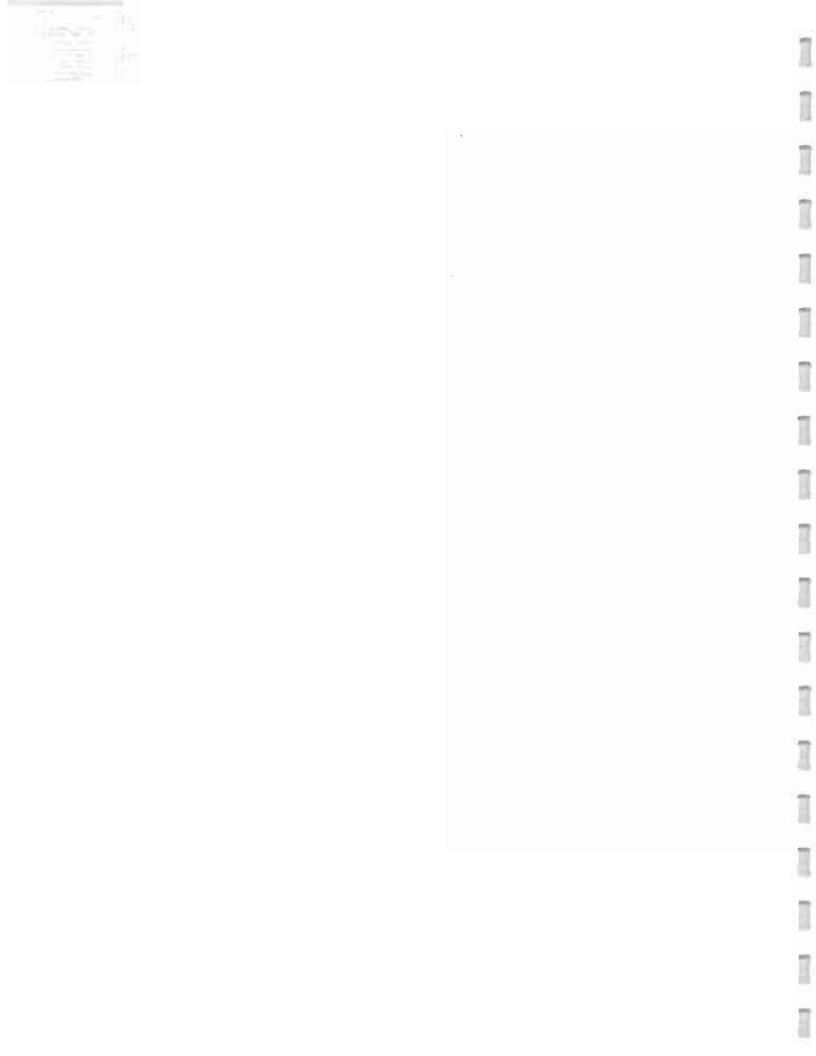


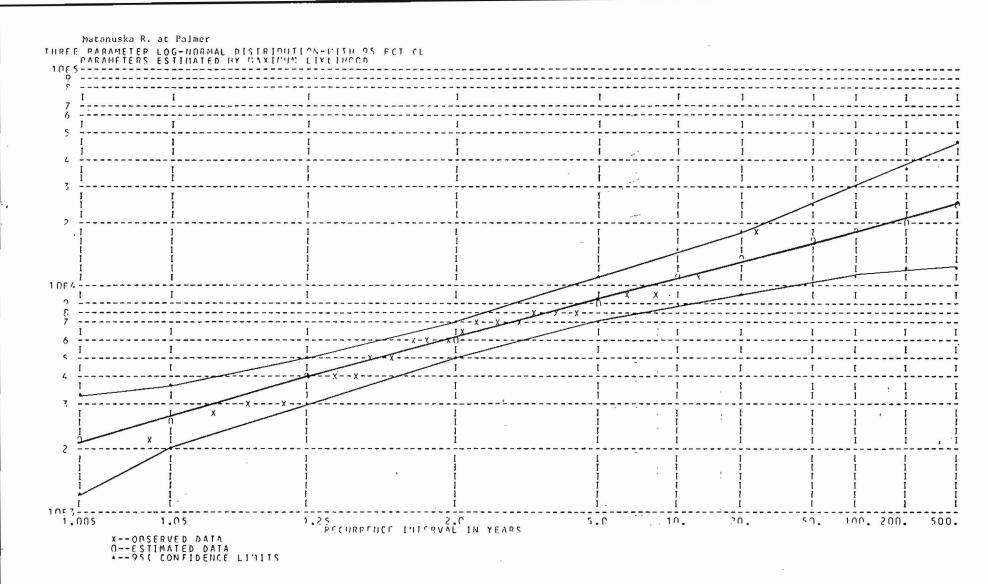
Prepared for:



OCT-MAY FLOOD FREQUENCY CURVE CARIBOU CREEK NEAR SUTTON



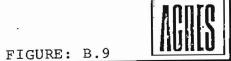




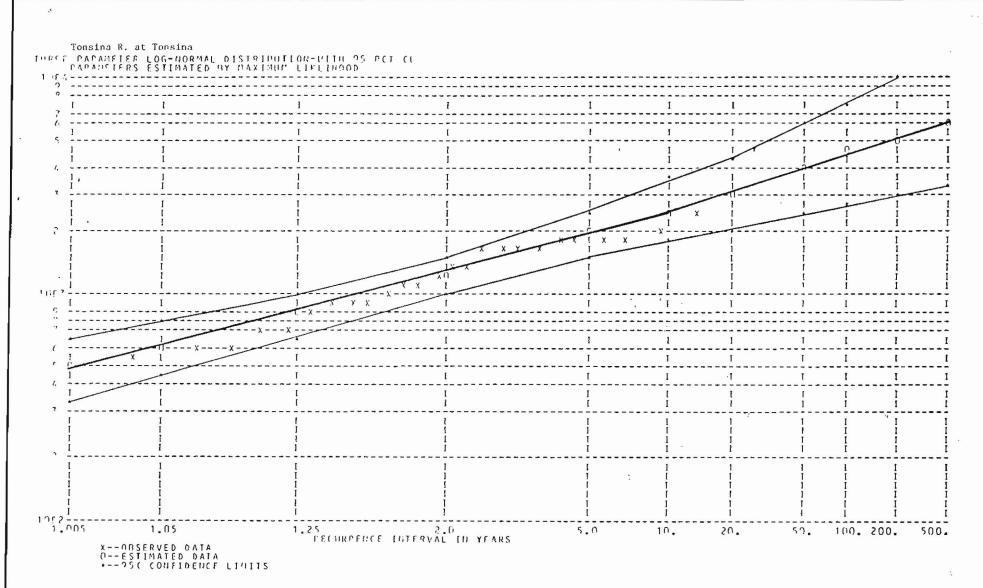
Prepared for:



OCT-MAY FLOOD FREQUENCY CURVE
MATANUSKA RIVER AT PALMER



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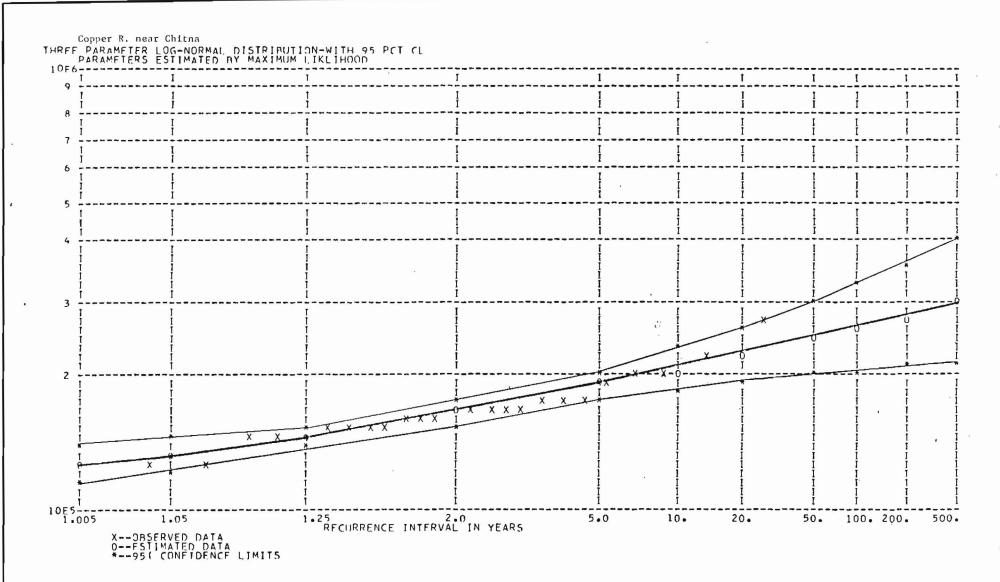
Prepared for:



OCT-MAY FLOOD FREQUENCY CURVE TONSINA RIVER AT TONSINA



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REM CONSULTANTS, INC.

OCT-MAY FLOOD FREQUENCY CURVE

COPPER RIVER NEAR CHITINA

Prepared for:



FIGURE: B.11

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ATTACHMENT C:

COMPARISON OF SUSITNA REGIONAL FLOOD PEAK
ESTIMATES WITH USGS METHODS

Susitna River at Gold Creek 1.25	Station Location	Return Period (Yrs.)	Single ¹ Station Estimate (CFS)	Susitna Regional Estimate (CFS)	USGS ² Area II Regional Estimate (CFS)	USGS ³ Cook Inlet Regional Estimate (CFS)
2	Susitna River at Gold Creek	1.25	37,100	37,700	48,700	=
10		2	49,500	49,000	59,200	43,800
Total Creek near Sutton 1.25 3,310 1,910 2,510 -		5	67,000	64,200	73,000	53,400
Caribou Creek near Sutton 1.25		10	79,000	74,500	83,400	55,300
Caribou Creek near Sutton 1.25		50	106,000	100,000	104,000	71,600
2		100	118,000	110,000	115,000	-
5	Caribou Creek near Sutton	1.25	3,310	1,910	2,510	-
10 7,170 3,760 6,280 4,200 50 9,480 5,050 8,810 6,210 100 10,500 5,570 10,500 - Matanuska River at Palmer 1.25 19,700 20,800 20,400 - 2 25,500 27,000 25,700 24,000 5 31,800 35,400 32,500 30,500 10 35,200 41,000 37,400 32,800 50 41,600 55,100 47,300 44,600 100 43,900 60,700 53,300 - Susitna River at Denali 1.25 14,000 12,400 15,900 - 2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		2	4,500	2,480	3,620	2,670
50 9,480 5,050 8,810 6,210 100 10,500 5,570 10,500 -		5	6,120	3,240	5,130	3,680
100 10,500 5,570 10,500 - Matanuska River at Palmer 1.25 19,700 20,800 20,400 - 2 25,500 27,000 25,700 24,000 5 31,800 35,400 32,500 30,500 10 35,200 41,000 37,400 32,800 50 41,600 55,100 47,300 44,600 100 43,900 60,700 53,300 - Susitna River at Denali 1.25 14,000 12,400 15,900 - 2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		10	7,170	3,760	6,280	4,200
Matanuska River at Palmer 1.25 19,700 20,800 20,400 - 2 25,500 27,000 25,700 24,000 5 31,800 35,400 32,500 30,500 10 35,200 41,000 37,400 32,800 50 41,600 55,100 47,300 44,600 100 43,900 60,700 53,300 - Susitna River at Denali 1.25 14,000 12,400 15,900 - 2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		50	9,480	5,050	8,810	6,210
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		100	10,500	5,570	10,500	-
5 31,800 35,400 32,500 30,500 10 35,200 41,000 37,400 32,800 50 41,600 55,100 47,300 44,600 100 43,900 60,700 53,300 - 2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600	Matanuska River at Palmer	1.25	19,700	20,800	20,400	-
10 35,200 41,000 37,400 32,800 50,900 35,600 50,900 35,600 47,000 50 41,000 37,400 32,800 32,800 50,900 37,400 32,800 50,900 50,900 35,600		2	25,500	27,000	25,700	24,000
50 41,600 55,100 47,300 44,600 100 43,900 60,700 53,300 - Susitna River at Denali 1.25 14,000 12,400 15,900 - 2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		5	31,800	35,400	32,500	30,500
100 43,900 60,700 53,300 - Susitna River at Denali 1.25 14,000 12,400 15,900 - 2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		10	35,200	41,000	37,400	32,800
Susitna River at Denali 1.25 14,000 12,400 15,900 - 2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		50	41,600	55,100	47,300	44,600
2 15,900 16,100 21,700 18,100 5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		100	43,900	60,700	53,300	-
5 20,200 21,100 30,000 23,600 10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600	Susitna River at Denali	1.25	14,000	12,400	15,900	-
10 24,100 24,500 36,500 25,700 50 36,600 32,800 50,900 35,600		2	15,900	16,100	21,700	18,100
50 36,600 32,800 50,900 35,600		5	20,200	21,100	30,000	23,600
		10	24,100	24,500	36,500	25,700
100 43,600 36,200 59,700 -		50	36,600	32,800	50,900	35,600
• • • • • • • • • • • • • • • • • • • •		100	43,600	36,200	59,700	-

ATTACHMENT C (Continued)

Station Location	Return Period (Yrs.)	Single ¹ Station Estimate (CFS)	Susitna Regional Estimate (CFS)	USGS ² Area II Regional Estimate (CFS)	USGS ³ Cook Inlet Regional Estimate (CFS)
MacLaren River near Paxson	1.25	4,450	3,760	4,880	_
	2	5,150	4,880	7,180	4,960
	5	6,800	6,390	10,600	6,790
	10	8,420	7,420	13,300	7,660
	50	13,700	9,960	19,800	11,200
	100	16,800	11,000	24,000	-
Susitna River near Cantwell	1.25	24,600	25,400	34,000	_
	2	32,300	33,000	42,500	29,800
	5	42,200	43,200	53,600	36,900
	10	48,400	50,100	61,900	38,400
	50	61,700	67,200	78,700	50,400
	100	67,100	74,200	88,500	-
Chulitna River near Talkeetna	1.25	33,100	28,500	28,200	-
	2	37,800	37,000	36,200	42,600
	5	45,100	48,500	47,100	53,400
	10	50,400	56,300	55,300	56,400
	50	63,100	75,500	72,600	75,200
	100	69,000	83,300	82,700	-
Talkeetna River near Talkeetna	1,25	20,300	20,900	30,000	-
	2	26,300	27,200	38,500	56,600
	5	38,500	35,600	50,100	71,400
	10	49,100	41,400	58,900	76,300
	50	80,100	55,500	77,400	103,000
	100	96,700	61,200	88,300	-

ATTACHMENT C (Continued)

Station Location	Return Period (Yrs.)	Single ¹ Station Estimate (CFS)	Susitna Regional Estimate (CFS)	USGS ² Area II Regional Estimate (CFS)	USGS ³ Cook Inlet Regional Estimate (CFS)
Montana Creek near Montana	1.25	2,040	2,520	1,130	-
	2	3,230	3,270	1,700	1,580
	5	4,670	4,280	2,500	2,220
	10	5,540	4,970	3,100	2,540
	50	7,290	6,670	4,490	3,770
	100	7,990	7,350	5,470	-
Skwentna River near Skwentna	1.25	26,800	23,600	15,700	-
	2	32,000	30,700	20,400	19,400
	5	38,900	40,200	26,500	24,600
	10	43,400	46,600	31,100	25,800
	50	53,200	62,500	40,400	34,300
	100	57,300	69,000	46,200	-
Tonsina River at Tonsina	1.25	3,540	4,820	2,130	- ·
	2	4,600	6,250	3,040	-
	5	6,040	8,190	4,260	-
	10	6,990	9,510	5,160	~
	50	9,090	12,800	7,130	_
	100	9,980	14,100	8,490	-
Copper River at Chitna	1.25	146,000	127,000	146,000	-
	2	164,000	164,000	165,000	_
	5	189,000	215,000	191,000	-
	10	206,000	250,000	210,000	-
	50	242,000	335,000	246,000	-
	100	257,000	370,000	265,000	-

Based on three parameter log normal distribution and shown to three significant figures

Lamke, R.D. (1979) Flood Characteristics of Alaskan Streams, USGS, Water Resources Investigation, 78-129

³ Freethey G.W. and D.R. Scully (1980) Water Resources of the Cook Inlet Basin, Alaska, USGS, Hydrological Investigations Atlas HA-620

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ATTACHMENT D

REVIEW OF FLOOD FREQUENCY ANALYSIS, CONDUCTED BY DR. ROBERT F. CARLSON, UNIVERSITY OF ALASKA, FAIRBANKS



UNIVERSITY OF ALASKA

MEMORANDUM REPORT

To: John Lawrence, Acres American Inc.

Steve Bredthauer, R&M Consultants, Inc.

From: Robert F. Carlson

Date: June 11, 1981

Subject: Susitna Hydroelectric Project

This report is in response to John D. Lawrence's letter of April 3, 1981, asking me to review the regional flood analysis conducted by R&M Consultants. The review is meant to include:

- a) Validity of gaging stations selected for the regional analysis and their representation of the hydrologic region analyzed.
- b) Adequacy and limitations of the analysis in development of regression expressions and the assumed 3-parameter log normal distribution.
- c) Validity of the shape, peak, and volume of the design flood hydrographs as developed.
- d) Discussion of review findings with Acres and R&M.

 The review is based on three reports which have been transmitted from R&M to Acres:
 - a) Two letters from James Aldrich to Ian Hutchinson, Re: Susitna Regional Flood Peak Analysis, Project No. 052305, dated October 27, and December 22, 1980.

b) A letter from Brent Drage to Ian Hutchison, Re: Susitna Regional Flood Analysis, Subtask 3.05, dated March 9, 1981.

The time and budget constraints of the review contract did not permit me to conduct an independent computation and verification of the values contained in the report. Therefore, the review is based on the material as presented by R&M. Nevertheless, several key values could be checked by comparison with published information.

Part (a). Validity of selected gaging stations.

The stations selected for regional analysis represent the transitional-continental climate zone of the Susitna basin. The Susitna basin's outstanding geographic characteristic is the combination of drainage from the south side of the Alaska range, the north side of the Tal-keenta mountains and the large flat boggy sub-basin north of Lake Louise. It is doubtful whether any other basin faithfully represents this type of hydrologic region. Of the twelve stations chosen for the regional analysis, the Susitna's hydrologic regime is best represented by:

- 1. Susitna at Gold Creek.
- Susitna near Denali.
- 3. Maclaren near Paxson.
- 4. Susitna near Cantwell.
- 5. Chulitna near Talkeetna.
- 6. Skewnta near Skewnta.
- 7. Montana near Montana.
- 8. Talkeetna near Talkeetna.

Stations that are not as closely representative are:

- Matanuska at Palmer.
 - 2. Tonsina at Tonsina.
 - Copper near Chitina.
 - 4. Caribou near Sutton.

The second group has been selected primarily on the basis of station distance from the Susitna basin and a different exposure to the coastal maritime climate. I do not feel the differences are great enough to warrant their exclusion from the regional analysis.

Part (b). Adequacy of the regression expressions and the assumed

3-parameter log normal distribution.

The assumed 3-parameter log normal distribution: Given the very sparse data base of the Susitna area streamflow records in combination with the great variability of extreme values, an attempt should be made to model or fit the sample data with a theoretical distribution of simple construction and few parameters. The analysis indicates that either the log-normal (LN) or the three-parameter log-normal (3PLN) distributions are nearly equally satisfactory. The 3PLN distribution has the advantage of further adjusting for the skew by inserting a lower truncation limit. I feel that, in view of the slight difference in predictability between the two distributions, the LN is preferred because of its simplicity. The analysis is not greatly compromised by using a 3PLN distribution, but the small reduction of the total deviation does

not seem to warrant the additional complexity. An inspection of rough frequency plots shows a good agreement between the fitted distribution and the sample points for both the LN and the 3PLN distributions. In summary, my preference for a frequency distribution would be the LN, but I do not see any great disadvantage in using the 3PLN.

The regression analysis: The regression analysis which attempts to predict the mean annual peak flow from watershed parameters has been carried out in a standard manner. A standard step-wise regression program has been used with up to twelve watershed parameters. Of the twelve, five parameters (drainage area, stream length, glacier area, mean annual precipitation, and mean annual snowfall) were shown to be statistically significant predictors of the mean annual instantaneous peak flow. Of these five, only the first three are directly related to basin parameters. The last two are estimated values and must be calculated from nearby gages or yet other basin parameters. Of the first three, the stream length and glacier area are potentially strongly related to drainage area. Larger areas are expected to be longer and, in the Susitna area, they should be strongly related to higher elevations and glaciers. Therefore, since drainage area is the most obvious predictor and far outweighs the others, it should be retained as the only independent predictor. The same reasoning holds for predicting the mean October-May peak flow.

There is one aspect of the regression analysis which is puzzling. The usual reason for an analysis of mean annual flood versus drainage

basin characteristics is to allow a regional estimation equation to be developed. Yet I can find no evidence of the completion of this equation or of its intended use. When using the 3PLN frequency model, three values of the sample must somehow be estimated (the mean, standard deviation, and skewness of the sample) or the three parameters of the distribution must be calculated (in this case the truncation level, the mean of the logs, and the standard deviation of the logs). I can find no estimates of the other two and only the Susitna at Gold Creek is used in the third memorandum.

My recommendation would be to estimate the mean annual flood (or May-October flood) from the drainage area and to use a pooled regional standard deviation and skewness to complete the description of the regional curve. The great sparseness of the data base must be constantly kept in mind and the most variable parameters pooled whenever possible.

In summary, unless some use is made of the regression exercise, which is not apparent in the three memorandums, I would not recommend its inclusion in the final report.

Part (c). Validity of the shape, peak, and volume of the design flood hydrograph.

The first step, a dimensionless comparison of the five largest hydrographs, seems to be straightforward and correct. I think the mean curve should be smoothed out a bit more, perhaps with a moving average method.

The time base (which determines the volume) is not clear. Also, whether the base flow was removed from the peak prior to the plot is not clearly indicated. I was not furnished with Enclosure 3 (the fifty peak hydrographs), so it is difficult for me to make an accurate assessment.

Although a flood peak versus volume relationship was established in Figure 7 of Memorandum 3, it is not clear whether the flood volumes were estimated from this graph or established independently through a separate frequency analysis. I assume a separate analysis has been made and the peaks and volumes, say for a 50-year return period, are matched to form the hydorgraph. That seems to be a logical procedure, but the volume under the stated flood peak hydrograph should equal the corresponding estimated flood volume. There is no clear indication that this comparison has been made.

In summary, the flood peak-volume analysis has been carried out in a satisfactory way, but the several points mentioned above should be cleared up for the final report.

Part (d). Discussion of review findings with Acres and R&M.

I had preliminary discussions with Brent Drage and Tim Renschler of R&M Consultants about the analysis. My comments were essentially the same as those I have presented here. In late May, I had a meeting with Steve Bredthauer of R&M Consultants during which we discussed the three memorandum reports in some detail. Again, the substance of my remarks has been presented here.

Part (e). Some additional items for discussion.

The instantaneous-daily flow ratio analysis seems to be logical and consistent. An average value of 1.1 for all stations would seem to be appropriate. The mean daily values should be used for the frequency analysis since they are stable and available in every case. Then the calculated ratio should be used when required.

A comparison of the predicted values of mean daily floods for the 100year return period from four sources compares reasonably well. The values are:

Lamke's U.S.G.S. Report	120,000 cfs
U.S. Corp's Interim Feasibility Report	110,000 cfs
R&M's Lognormal Estimate	112,625 cfs
R&M's 3-Parameter Lognormal	117.782 cfs

These values are reasonably close, given the general vagueness which is a part of every flood frequency analysis. One can be fairly sure that the flood frequency analysis has been carried out correctly.

Part (f). Concluding remarks.

This memorandum report completes my review of the Susitna flood frequency report. I hope the comments will be useful for understanding the application of the frequency analysis and for preparation of this final report. I will be willing to assist in any additional analysis which may be required or to answer questions about my comments.

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