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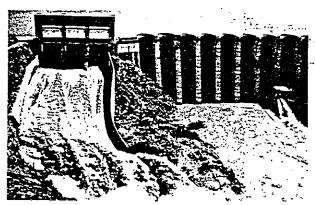
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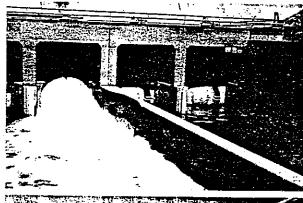


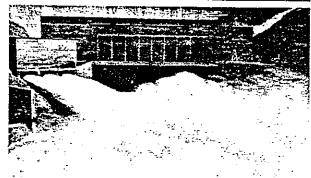
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EXCERPT FROM

AIR-WATER FLOW IN YDRAULIC STRUCTURES

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UNITED STATES DEPARTMENT OF THE INTERIOR WATER AND POWER RESOURCES SERVICE As the Nation's principal conservation agency, the Department of the Interior has the responsibility for most of our nationally owned public lands and natural resources, protecting our fish and wildlife, preserving the environmental and cultural values of our national parks and historical places, and providing for the enjoyment of life through outdoor recreation. The Department assesses our energy and mineral interests of all our people. The Department also has a major responsibility for American Indian reservation communities and for people who live in Island Territories under U.S. administration.

ENGINEERING MONOGRAPHS are published in limited editions for the technical staff of the Water and Power Resources Service and interested technical circles in Government and private agencies. Their purpose is to record developments, innovations, and progress in the engineering and scientific techniques and practices which are used in the planning, design, construction, and operation of water and power structures and equipment.

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Preface

The material assembled in this report is the result of studies extending over many years by a large number of engineers. Ellis Picket at the U.S. Army Engineer Waterways Experiment Station in Vicksburg, Mississippi, supplied a reference list dealing with air-water problems. Personnel of the Water and Power Resources Service E&R Center. Water Conveyance Branch made their files and drawing on air design criteria in pipelines available for publication in this report. Prior to publication, the report was reviewed by Ellis Pickett and Ted Albrecht with the U.S. Army Engineers; and by engineers in the Dams, Mechanical, and Water Conveyance Branches, E&R Center, Water and Power Resources Service. The many constructive comments by these individuals and the assistance of Richard Walters who provided continuity and technical editing is greatly appreciated.

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Letter Symbols and Quantities

Symbol	Quantity	Symbol	Quantity
\boldsymbol{A}	Cross sectional area of water prism	d	Flow depth
$\stackrel{\scriptstyle II}{A_a}$	Cross sectional area of airflow	d_b	Bulked flow depth
7. a	passage	d_e	Deflector height
A_{c}	Cross sectional area of air core in a	d_n	Nappe thickness
1*C	vertical shaft	d_o	Orifice diameter
A_d	Cross sectional area of conduit	d_t	Total depth of underlying and air
A_o	Orifice area		free zones
$\stackrel{A_0}{A_p}$	Cross sectional area of penstock	d_{95}	Bubble diameter for which 95
$\stackrel{I_p}{A_v}$	Cross sectional area of vent		percent of the air, by volume, is
2×0 8	Ratio of bubble terminal velocity in		contained in bubbles of this
	turbulent flow to terminal velocity		diameter or smaller
	in still water	$oldsymbol{E}$	Relative width of the frequency
& 0	Mean air distribution function		spectrum
. a ₁	Mean air distribution constant	exp	Napierian logarithm equal to
B	Width of rectangular chute		2.71828, approximately
b	Width of flow channel	f	Darcy-Weisbach friction factor
b_n	Nappe width	$\overset{f}{G}$	Gate opening
b_s	Empirical coefficient accounting for	G_{g}	Mass velocity of gas
O _S	sand grain roughness	$oldsymbol{G_i}$	Mass velocity of liquid
С	Air concentration	g	Gravitational constant (acceleration)
C_a	Actual air concentration	\check{H}	Hydraulic radius of prototype air
C_b	Drag coefficient on a bubble		vent
C_d	Discharge coefficient based on 100	H_f	Fall height of a water jet
· Oa	percent gate opening	H_m	Head across orifice
C_f .	Local loss coefficient	H_n	Net head across turbine
C_l	Air concentration at $d_t/2$	H_o	Distance from channel invert to
C_m	Air concentration measured by a	•	energy grade line
Οm	pitot tube sampler	H_t	Total potential and kinetic energy
C_o	Orifice discharge coefficient	h	Mean wave height
C_s	Drag coefficient on a sphere	$h_{\scriptscriptstyle R}$	Height of airflow passage
C_t	Air concentration at the bottom of	h_f	Distance from inlet to the water
J,	the mixing zone	-	level in the vertical shaft
\overline{C}	Mean air concentration	h_{l}	Head loss per unit length
c	Waterhammer wave celerity	h_m	Head across manometer
\check{D}	Conduit diameter	h_w	Allowable head rise in penstock
$\widetilde{D_b}$	Smaller dimension of a rectangular	K_e	Entrance loss
20	conduit	K_s	Singular (form) loss
. D_d	Diameter of water drop	k	
D_e^a	Equivalent bubble diameter		equal to 0.4
D_s	Larger dimension of a rectangular	k_r	Coefficient of roughness
- / ₅	conduit	k_s	Sand grain roughness
	~ · · · · · · · · · · · · · · · · · · ·		more a productive

LETTER SYMBOLS and QUANTITIES—Continued

Symbol	Quantity	Symbol	Quantity
$_{L_{c}}^{L}$	Length of conduit or vent Distance to start of self-aeration	r_s	Relative roughness of conduit (rugosity to diameter ratio)
L_r	Prototype to model scale ratio	\boldsymbol{S}	Submergence depth
L_s	Distance between stiffener rings	S_o	Pipe slope
\check{M}	Unit mass	S_f	Slope of energy grade line
<i>M</i> _o	Maximum difference in elevation between a wave crest and the mean water level	s Sw	Root-mean-square value of wave height distribution Root-mean-square value of water
m	Air concentration distribution	s_w	surface distribution
	coefficient	T	Top width of flow passage
N	Safety factor	$\frac{1}{t}$	Pipe wall thickness
n	Manning's roughness coefficient	$oldsymbol{\dot{U}}$	Free stream velocity
n_v	Velocity distribution power-law coefficient	U_d	Velocity of water drop relative to air velocity
\boldsymbol{P}	Energy dissipated	U_j	Water jet velocity
$P_{\mathcal{g}}$	Normal distribution function	u u	Local air velocity
P_h°	Probability that the wave height is	\overline{v}	Mean flow velocity
	equal to given height	V_f	Terminal velocity of bubbles
P_w	Probability that the water surface	,	in turbulent flow
	is equal to or greater than the	V_i	Nappe velocity at impact
	given elevation	V_m	Minimum velocity required to
p	Pressure intensity		entrain air
$p_{_B}$	Allowable internal pressure	V_o	Maximum water surface velocity .
p_{atm}	Atmospheric pressure	V_s	Terminal velocity of bubbles in
p_c	Collapse pressure		slug flow
p_{in}	Internal pressure	V_t	Terminal velocity of bubbles in
p_n	Nappe perimeter		still water
Q	Discharge	W	Wetted perimeter
$Q_{\scriptscriptstyle B}$	Volume flowrate of air	x	Distance from start of boundary
Q_c	Critical discharge		layer growth
Q_r	Discharge from reservoir	y	Distance normal to channel bottom
$Q_{m{w}}$	Volume flowrate of water		(flow depth)
\boldsymbol{q}	Unit discharge	У в	Distance from water surface
q_{z}	Insufflation rate of air per unit surface area	Ус Уе	Conjugate depth Effective depth
\boldsymbol{R}	Bubble radius	\mathbf{y}_{k}	Critical depth
R_b	Equivalent bubble radius	y'	Normal distance to the bottom of
R_c	Radius of curvature of the bubble cap		the mixing zone
R_{j}	Thickness of annular jet	z	Elevation
r	Water jet radius		

LETTER SYMBOLS and QUANTITIES—Continued

Symbo	ol	Quantity	Symbo	l Quantity		•
α	alpha	Angle chute invert makes with horizontal	E	Eötvös number	=	Y1)2
β	beta	Ratio of volumetric airflow rate to waterflow rate	\mathbf{E}_{u}	Euler number	=	$\frac{\Delta p}{e^{1/2}}$
γ	gamma	Specific force of water		.		12
ó	delta	Boundary layer thickness	F	Froude number	=	$(gD)^{1/2}$
£	epsilon	Mass transfer coefficient of bubbles	· P	Prandtl velocity		.8.71
ζ	zeta	Air concentration distribution constant	•	ratio	=	$\frac{J'}{(\tau_0/\varrho)^{1/2}}$
η.	eta	Normalized wave height	т.	n e ai L		$h_a^{-2}\left(dp/dx\right)$
θ	theta	Void fraction	\mathbf{P}_{o}	Poiseuille number	=	$\frac{2}{\mu V}$
x	kappa	Gas constant		•		-μν
λ	lambda	Density ratio	R	Reynolds number	=	$V\underline{D}$
μ	mu	Dynamic viscosity		•		ν
$\mu_{\mathbf{z}}$		Dynamic viscosity of air	n	D'. D 11		
μ_{w}		Dynamic viscosity of water	\mathbf{R}_{x}	Distance Reynolds number	<u></u>	$v_{\underline{x}}$
ν	nu	Kinematic viscosity		nunsser		ν
ν_f	:	Water viscosity Ratio of the circumference				• •
'n	pi	of any circle to its radius, 3.14159	W	Weber number	=	$(\alpha/\varrho \overline{D})^{1/2}$
Q	rho	Density		•		
Q _B		Air density		•		
Qw		Water density				
ϱ_g		Gas density	•	•		
Qi		Liquid density		•		
ϱ_m		Density of manometer fluid				
σ	sigma	Interfacial surface tension	•			
τ_o	tau	Wall shear stress				
τ_j	•	Shear stress at water jet				
v_{atm}	upsilon	Specific volume of air at atmospheric pressure				
υ _*	•	· Shear velocity				
Ψ	psi	Multicomponent flow parameter				
ω	omega	Volume of gas bubble	•			
$\omega_{\scriptscriptstyle B}$		Volume of air				
ω_w		Volume of water				
				ŀ		
			•			

Infinity

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Introduction

In many engineering projects a strong interaction developes between the water flowing through a structure and the air which is adjacent to the moving water. Sometimes the interaction produces beneficial effects. However, more often than not, the effects are not beneficial and the remedial action required to reduce the effects can be costly.

Cases in which air-water interaction develop include:

- Open channels with fast flowing water that require depths adequate to contain the air which is entrained within the water
- Morning-glory spillways that must have a capacity to convey the design flood and its entrained air
- Vertical shafts that entrain large quantities of air at small water discharges
- Measuring weirs that need adequate ventilation to prevent false readings and to eliminate surging
- Outlet gates that require adequate aeration to prevent the development of low pressures—which can lead to cavitation damage
- Emergency gates at penstock entrances that require ventilation to prevent excessive negative internal pressures during draining or emergency gate closures

- Sag pipes (inverted siphons)¹ that can be damaged due to blowback of entrained air
- Long pipelines that require air release and vacuum relief valves

From these cases it is noted that air-water flows can be generalized into three basic flow types:

- 1. Air-water flows in open channels,
- 2. Air-water flows in closed conduits, and
- 3. Free-fall water flows.

The first type usually is called air-entraining flow because air is entrained into the water mass. The second basic flow type generally is referred to as air-demand. The term air-demand is both misleading and technically incorrect, since an air vent does not demand air any more than an open valve demands water. However, since the term has been in common use for over 20 years, efforts to improve the nomenclature seem rather futile. The third type is referred also to as air-entraining flow.

[&]quot;siphon, inverted—A pipe line crossing over a depression or under a highway, railroad, canal, etc. The term is common but inappropriate, as no siphonic action is involved. The suggested term, sag pipe, is very expressive and appropriate." Nomenclature for Hydraulics, Comm. on Hyd. Str., Hyd. Div., ASCE, 1962.

Purpose and Application

The purpose of this report is to summarize the work that has been done on air-entrainment and air-demand regarding the most recent theories and to suggest ways in which the results can be applied to design. The intent was to produce a concise reference of material from which design manuals, nomographs, and charts for specific applications could be prepared.

Although many generalizations of the data can be made, some types of flow conditions that are encountered in practice can be treated only by individual studies with physical models. These cases are identified when they occur.

Additional studies are needed in many areas. Some of the most critical areas requiring further research include the following:

- Effects of turbulence and air concentration on bubble dynamics
- Fluid dynamics in the developing aeration regime of free-surface flow
- Effects of hydraulic and conduit properties on probabilistic description of water surface in free-surface, high-velocity flow
- Effect of pressure gradients on air flow in partially-filled, closed conduits
- Bubble motion in closed-conduit flows for conduit slopes exceeding 45-degrees
- Effects of ambient pressure levels on cavitation characteristics of gates and valves discharging into a closed conduit
- Interaction between the air and a free jet

Summary and Conclusions

Methods have been developed to predict the mean air concentration and the concentration distribution with open channel flow. A new description of the free water surface in high velocity flow is proposed which more accurately represents actual conditions in high velocity flow. The effect of air entrainment on the performance of a stilling basin can be estimated using a bulked flow concept. A computer program (app. II) is presented with which the mean air concentration in steep chutes and spillways can be estimated.

With exception of a falling-water surface and decreasing flow in pipelines, closed conduit flows require model studies. When properly conducted and analyzed, model studies will yield accurate data for estimating air-flow rates. Experimental methods are discussed. A computer program (app. III) is presented which can be used to predict the airflow rate with a falling-water surface. Design charts are presented for sizing air relief valves and vacuum valves on pipelines.

The airflow rate in vertical shafts was found to be extremely dependent upon the flow conditions at the shaft inlet. Equations are included for estimating the airflow rate having various inlet conditions.

Factors influencing the airflow rate around free falling jets are discussed. This area is identified as one needing additional research. Equations are presented from which the air entraining characteristics of a jet entering a pool can be estimated.