

SUSITNA HYDROELECTRIC PROJECT

FY83 PROPOSED GEOTECHNICAL  
EXPLORATORY PROGRAM

DRAFT

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ALASKA POWER AUTHORITY  
SUSITNA HYDROELECTRIC PROJECT  
FY83 PROPOSED GEOTECHNICAL EXPLORATORY PROGRAM

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## 1 - INTRODUCTION

This report details the proposed FY83 (July 1, 1982 through June 30, 1983) Task 5 Geotechnical program for the Susitna project. The scope of the project is based on Acres' and others' previous work at the sites, as well as directives set forth by the Power Authority's and Acres' consultants review panel. The level of effort for the FY83 is mandated by scheduling and logistic constraint and budget. Because the Watana site will be the first site developed, all FY83 activities will be directed toward that site.

The FY83 field program will be performed in three phases. Phase I will commence on July 1 and continue through September 30; Phase II will begin in December - January and continue through March - April; and Phase III will commence in May - June and continue through the end of fiscal year. Subsequent phases of study will be a continuance of Phase III activities.

This report is intended to provide a clear understanding of the scope, objective, methodology of approach, schedule, and costs for the FY83 activities. It is not the intent of this report to "cast-in-concrete" the proposed field activities, but rather to set forth general guidelines to be followed. To expedite the project, all field data will be reduced and interpreted in the field. This will assist the field personnel in scoping and revising the field studies as required to meet the objectives of the project.

Work performed during Phase I will be used in detailing the subsequent phases of work. The four principal areas that will be investigated during FY83 are:

- Relict Channels;
- Borrow Area D;
- Borrow Areas E, and I; and
- Damsite.

The FY83 program will involve a multidisciplinary approach to include, geologic mapping, geophysical explorations, subsurface investigation, and laboratory testing. The following sections detail the scope, cost, and schedule for Phase I and II activities. The scope of Phase III will be dependent on information obtained in Phases I and II, as well as budgetary constraints.

## 3 - DAMSITE INVESTIGATION

### 2.1 - General

The FY83 damsite investigation will be a continuation of the previous investigation performed during 1980-81 programs. The principal objectives are to:

- Confirm geologic mapping and interpretations previously performed onsite;
- Determine depth of river alluvium beneath the proposed main dam and cofferdam; and
- Determine rock condition in areas of proposed major civil structures.

The damsite investigation will consist of geologic mapping during Phase I; seismic refraction during Phase I and II; subsurface drilling during Phase II and III. Details of the damsite exploration plans are shown in Figure 2.1 and described on Tables 2.1 through 2.3. A schedule of field activities is contained in Section 8.

### 2.2 - Civil Structures

#### (a) Main Dam and Cofferdam Riverbed Foundations

##### (i) Objectives

The objectives of this investigation will be to complete explorations for riverbed foundation conditions for cofferdams and main dam. Explorations will examine both alluvial and bedrock conditions beneath the river and on the adjacent abutments. Data from this task will be used in developing the general arrangement and design of the cofferdams.

##### (ii) Discussion and Approach

The alluvial deposits of the Susitna River consist of gravel, cobbles, and boulders with a sand matrix. The thickness and type of

material, as well as bedrock conditions, is known at only several locations. Development of the general arrangement and design of cofferdams and the main dam will require detailed data on the following: thickness and types of alluvial material, frozen layers, top of bedrock surface, bedrock lithology and structure, and depth to sound rock. The FY83 investigation to obtain this information will consist of geologic mapping, geophysical explorations, and drilling. Geologic mapping, which will be performed during Phase I, will consist of two parts: first, examination of the geology in the immediate area of the proposed cofferdam locations; and, second, mapping of geologic structures on the abutments which may affect dam and cofferdam locations and design. The latter work will be discussed in a subsequent Section. Geologic mapping of the cofferdam sites will consist of examining alluvial and bedrock conditions in the immediate area of the planned locations.

During the winter a seismic refraction survey on the frozen river surface will be performed. The surveys will run from "The Fins" to downstream of the "Fingerbuster" to define the top of bedrock surface and alluvial thickness. The seismic lines will be arranged in a grid pattern parallel to the cofferdam axis and along the center of the river (Figure 2.1 and Table 2.2). Results of this survey will be the basis for selecting river borehole locations. Boreholes will be drilled during the winter from the surface of the frozen river to confirm the seismic line data and to sample alluvium and bedrock. A "Becker" type rig, capable of drilling 400-600 feet deep in alluvial material, will be used for the drilling and sampling. Drilling will consist of both plug holes and sampled holes (Table 2.3). Plug holes will be drilled to confirm fluvial thickness and depth to sound bedrock. Sampled holes will continuously sample the alluvium and bedrock. Rock drilling, for both plug and sampled holes, will continue until a minimum of 10 feet of sound bedrock is encountered. A minimum of two of the holes beneath the core of the proposed dam will be core drilled to a depth of 50-100 feet and pressure tested to determine rock permeability.



(iii) Schedule

Geologic mapping for the cofferdam locations will be done by a team of two geologists. Because of the priority, this mapping will be done at the onset of the field season in Phase I. During the winter when ice conditions on the river are suitable, approximately 9,000 feet of seismic lines will be run along the axis of the river and an additional 4,000 linear feet in the cofferdam areas (Table 2.2). Drilling of the river alluvium will commence after completion of the seismic lines. A total of 5 drill holes are planned for each cofferdam site: two sample holes and three plug holes. Total drill footage for the cofferdam sites is estimated to be 1,000 feet. An additional 5 drill holes will be drilled beneath the proposed main dam.

(b) Portals

Portals include the upstream diversion tunnel portals and downstream diversion, spillway, access, and tailrace tunnel portals.

(i) Objectives

The objectives of the portal investigation will be to complete explorations for portal locations, orientations, and support requirements for upstream and downstream portals. Explorations will examine and define overburden materials and bedrock conditions.

(ii) Discussion and Approach

The primary geologic structures which may have an affect on the upstream and downstream portal design are the geologic structures referred to as "The Fins" and the "Fingerbuster", respectively (Figure 2.2). These features will be the main focus of data gathering. Both areas are characterized by northwest and north trending shear, fracture, and alteration zones. Bedrock is well exposed in "The Fins" in a series of cliff faces. Outcrops are not as common in the "Fingerbuster" where talus covers much of the area.

The investigation will examine and describe the type and thickness of overburden material, bedrock lithology and structure, depth to sound bedrock, rock permeability, ground water table, and areas of potential slope instability. Explorations for the portals will consist of geologic mapping, seismic refraction surveys, and drilling. Explorations will be more detailed and concentrated than during previous investigations. Geologic mapping will precede other forms of exploration in the summer of FY83, and will form the basis for locating seismic lines and drill holes. Geologic mapping will consist of identifying and describing the following: overburden material and extent, bedrock outcrop extent, bedrock lithology, joints and joint spacings, shear, fracture, and alteration zones, and areas of potential slope instability. For the upstream portal, mapping will be done in an area approximately 500 feet upstream and downstream from the proposed portal locations and between river level and elevation 1,800 feet. Mapping for the downstream portals will cover an area from the toe of the proposed dam to the "Finger-buster" shear zone, approximately 1,500 feet, and between river level and elevation 2,000 feet (Figure 2.2). Data from the mapping will be plotted on maps and sections of suitable scale and on joint stereo plots. This data will be interpreted and used for further explorations and design requirements.

A seismic refraction survey will be run in the downstream portal area to define overburden thickness structures, and depth to sound bedrock. Seismic lines will be run in a grid pattern based on the results of geologic mapping (Figure 2.1). An estimated 4,500 feet of seismic lines will be run. A 1,000-foot seismic line is planned at this time for the upstream portal. This line will be run on the frozen river surface immediately offshore of "The Fins" structure.

Boreholes will be drilled in both portal areas to determine type and thickness of overburden material, subsurface geologic conditions, rock permeability and ground water conditions. Drilling will consist of approximately 6 to 10 predominantly angled boreholes to

various depths during Phase III. Total drill footage is estimated at 2,000 linear feet. Bedrock will be continuously cored using NQ-size core barrels. Core samples will be logged for lithology and structure. Selected samples will be tested for unconfined compressive strength using a point load tester. Following completion of the drilling, joint and fracture orientations at depth will be determined using a borehole camera. Water pressure testing will be done over the length of the borehole to determine rock permeability. One borehole in the upstream portal area and two boreholes in the downstream portal area will be instrumented with piezometers for ground water monitoring.

(iii) Schedule.

Geologic mapping will begin following completion of cofferdam site mapping. One, two-man team of geologists will be used for mapping the portal areas. Seismic surveys will be run following geologic mapping. Diamond core drilling will be done during Phase III.

## 2.3 - Geologic Structures

(a) Shear, Fracture, and Alteration Zones

Shear, fracture, and alteration zones are the northwest and north trending geologic features identified during the 1980-81 explorations. This aspect of the exploration does not include "The Fins", "Fingerbuster", and geologic features GF7 and GF8 which will be investigated during subsequent phases of exploration Figure 2.2)(1).

(i) Objectives

The objectives of the geologic structure investigation is to provide adequate geological and geotechnical information needed for developing the general arrangements. Explorations will be directed to

determining the nature and extent of these geologic features at depth.

(ii) Discussion

Northwest and north trending geologic structures which include shear, fracture, and alteration zones were identified during previous investigations<sup>(1)</sup>. These features were tentatively correlated across the site on the surface and to depth in boreholes. The FY83 investigation will attempt to confirm these geologic interpretation.

Principal attention will be directed to the right abutment where the majority of civil structures are tentatively proposed.

Areas requiring further investigation are geologic features GF2, GF3, and GF4, which were defined in the 1980-81 program as potentially projecting into the proposed powerhouse intake area (Figure 2.2).

Approach

Explorations for the northwest and north trending geologic structures will consist initially of geologic mapping and seismic refraction surveys followed by diamond core drilling. Geologic mapping will precede other forms of exploration and will be the basis for locating seismic lines and boreholes. Geologic mapping will consist of identifying and describing the following: type of geologic structure, orientation and extent, degree of weathering or alteration and joint sets, and spacing. Geologic data will be plotted on maps and sections at a suitable scale. This data will be interpreted in the field and used as the basis for planning future explorations. Mapping will consist primarily of traversing along the extent of the structures and accurately locating its position. Cross traverses will be run across the abutments, as necessary. Most of the traverses will be done on the right abutment between

"The Fins" and "Fingerbuster" shear zones from river level to elevation 2,300 feet. On the left abutment, mapping will extend from geologic feature GF1B to GF6B and from river level to elevation 2,000 feet (Figure 2.1).

Seismic refraction lines will be run on both abutments to define the location of low velocity zones related to shear, fracture, and alteration zones as well as thickness of overburden materials, and depth to sound bedrock (Figure 2.1). On the right abutment, one seismic line will be run from "The Fins" area parallel to the slope at about elevation 1,800 feet to the "Fingerbuster" shear zone. This line will cross the entire abutment and all major geologic structures. An additional seismic line at approximately elevation 2,050 feet will connect seismic lines SL81-15 and SW-2 to complete coverage at this elevation. An estimated 6,000 linear feet of seismic lines will be run on this abutment. On the left abutment, one seismic line will be run eastward from the end of SL81-20 between elevation 2,000 and 2,050 feet. This line will help define the extent of geologic features in GF3 and GF4. Line length is approximately 2,000 linear feet. Seismic velocities will be plotted on maps and sections at the same scale as geologic data. Correlations will be made where possible between low seismic velocity zones and mapped surface expressions of geologic structures.

Based on the interpretation of geologic and seismic data, boreholes will be drilled across various geologic structures to verify their presence and define their subsurface characteristics. Data will include joint spacing and, where possible, orientation, rock permeability and strength, and ground water conditions. Drilling will consist of approximately 10 angled boreholes to an average depth of 500 feet. Locations of these borings will be based on Phase I and II activities. Boreholes will be drilled on both abutments and beneath the river. Bedrock will be continuously cored using NQ-size wireline core barrels. Core samples will be logged for lithology and structure. Selected samples will be tested for unconfined compressive strength using a point load tester. Following

completion, fracture and joint orientation at depth will be determined using a borehole camera. Water pressure testing will be done over the length of the borehole to determine rock permeability. Selected boreholes will be instrumented with piezometers for ground water monitoring.

(iv) Schedule

Geologic mapping will begin following portal area mapping. One two-man team of geologists will be used for mapping on the abutments. Diamond core drilling will be done during Phase III following completion of portal area drilling.

(b) Pluton Structure

(i) Objectives

The objective of this investigation is to establish the geologic model for the damsite area and quarry sources for general site arrangements. Explorations will examine and define the extent of the diorite pluton, the nature of its contacts with adjacent rock types, and its structural relationships<sup>(1)</sup>.

(ii) Discussion

The Watana damsite is underlain by a diorite pluton which has intruded into surrounding metasedimentary. West of the damsite, the pluton is overlain by andesite porphyry which, in turn, appears to be overlain by volcanoclastic rocks. The andesite porphyry has intruded the diorite pluton. The contact between the diorite and andesite porphyry is generally fractured with minor, local shearing. The andesite porphyry immediately above the contact is generally highly fractured, however, the underlying diorite is generally unfractured. The eastern boundary of the pluton has been projected to cross through the reservoir area east of the damsite; however,



this contact has not been adequately defined. The pluton is crosscut by shear, fracture, and alteration zones. The origin of these zones, whether due to regional tectonic forces, local pluton related forces, or a combination of the two, has not been determined.

Understanding of the lithology, structure, and contacts of the pluton and surrounding rocks requires the preparation of a geologic model of the development of the diorite pluton and surrounding rocks in the damsite area. The geologic model can be used to predict the nature and extent of various lithologies and structures where scarce data exists.

(iii) Approach

Explorations for the development of a geologic model for pluton structure will consist of geologic mapping based on previous investigations. Mapping will consist of describing and defining the limits of diorite pluton, nature of lithologic contacts, nature of geologic structures at the contacts, and extent of geologic structures away from the main damsite. Geologic mapping will be done upstream and downstream from the damsite along the Susitna River, on Tsusena and Deadman Creeks, in proposed Quarry A, and any other suitable locations based on aerial reconnaissance. Geologic mapping for this investigation will not require the level of detail or control as that done for the portals, cofferdams or shear, fracture, and alteration zones. Detail airphoto interpretation will be done prior to mapping to identify locations to be mapped.

In addition, detail petrographics studies will be performed on various rock types collected during the mapping program to determine mineralogy, lithology, and the sequence of geologic events.

Geologic mapping for pluton structures will require a team of two geologists during Phase I.

(c) "The Fins" and "Fingerbuster"

(i) Objectives

The objective of this investigation is to complete explorations of these geologic features for general arrangements and design of underground civil structures. Explorations will examine and define the surface and subsurface nature and extent of these features.

(ii) Discussion and Approach

"The Fins" and "Fingerbuster" bound the damsite on the upstream and downstream sides, respectively (Figure 2.2). Both features contain major northwest and north trending shears, fractures, and alteration zones which would have a significant impact on costs for any civil structures located within them. At present, these structures have been identified on the right abutment and tentatively projected, based on correlation with boreholes, seismic data, and topography. This investigation will attempt to confirm these correlations and define and describe, in detail, the surface and subsurface characteristics of these features. Explorations will consist initially of geologic mapping followed by seismic refraction surveys and diamond core drilling. Geologic mapping will be the basis for locating seismic lines and boreholes. Mapping will consist of identifying and describing the following: type of geologic structure, orientation and extent, degree of weathering or alteration, and joint sets, continuity, and spacing. Geologic data will be plotted on maps and sections at suitable scales. This data will be interpreted in the field for use in planning further explorations. Geologic mapping will consist of tracing the extent of these features by ground traverses and aerial reconnaissance. Mapping on the north bank will extend from the Susitna River to Tsusena Creek. On the south bank, limited outcrops will restrict mapping to within about one mile of the river.



Seismic refraction lines will be run in both "The Fins" and "Fingerbuster" areas to define the extent of low seismic velocity zones related to these features, thickness of overburden, and depth to sound bedrock. The amount and location of seismic lines is dependent on the results of geologic mapping. An estimated 8,000 linear feet for "The Fins" and 4,000 linear feet for the "Fingerbuster" are planned (Figure 2.1).

Geologic mapping of "The Fins" and "Fingerbuster" is planned during Phase I with two geologists forming a team.

## 2.4 - Miscellaneous Investigations

### (a) Instrumentation of Existing Boreholes

#### (i) Objectives

The objective of this exploration is to gain additional data on ground water and permafrost conditions in the bedrock at the main damsite for design and construction activities.

#### (ii) Discussion and Approach

To complete the instrumentation program at the damsite requires additional piezometers and thermistors to be installed. At present, thermistors are located on the right abutment in BH-3 and BH-6. It is proposed that a thermistor string be installed in BH-8 (drilled in 1981) to evaluate permafrost conditions on the left abutment. Thermistor construction would be the same as the existing units with 16 thermistor points along a 250 foot cable.

Two piezometer installations are proposed for special conditions in Boreholes BH-4 and BH-12. A pneumatic piezometer tip will be installed at the elevation of the powerhouse in BH-4 to determine

water pressure conditions at this depth. In BH-12, a pneumatic piezometer tip will be installed below the artesian aquifer at about 400 feet.

To install the piezometers and thermistors, boreholes BH-4, BH-8, and BH-12 will have to be reamed out to the required depths to allow placement. Approximately 1,600 feet of reaming will be required.

(iii) Schedule

This work will be undertaken during Phase III.

(b) Survey Control

(i) Objective

The objective of survey control is to establish adequate ground control for use in damsite investigations.

(ii) Approach

Upon commencing work in July, a survey grid will be established on both abutments and along the river for ground control. The grid will be on an average of 500 foot spacing with permanent markers established at designed locations. The grid will be tied into the State of Alaska grid system.

All field exploration performed on the damsite will be controlled by the surveyed grid.

(iii) Schedule

Work on establishing the grid will be undertaken in the early part of Phase 1.

**TABLE 2.1: FY83 EXPLORATORY PROGRAM - PHASE 1  
WATANA DAMSITE GEOPHYSICAL SURVEY**

<u>Seismic Refraction Line No.*</u>	<u>Location**</u>	<u>Approximate Length (ft)</u>	<u>Purpose</u>
SL82-D	Downstream Portals, BH-6 towards BH-2	2,200	Depth to bedrock, overburden thickness.
SL82-E	Downstream Portals, DM-C to SW1	1,100	Overburden thickness, bedrock conditions.
SL82-F	Downstream Portals, SW-2 to river	1,100	Overburden thickness, bedrock conditions.
SL82-G	Flip Bucket	1,100	Overburden thickness, bedrock conditions in "Fingerbuster".
SL82-H	Downstream Portals, mid slope	2,200	Overburden thickness, bedrock conditions, NW and N shears.
SL82-I	DH-9 to SL80-2	1,100	Overburden thickness, bedrock conditions, NW and N shears.
SL82-J	Spillway	4,400	Overburden thickness, bedrock conditions, NW and N shears.
SL82-Ka	Right Abutment	2,200	Overburden thickness, bedrock conditions, NW and N shears.
SL82-Kb	Upstream Right Abutment	1,100	"The Fins," NW and N shears.
SL82-L	Powerhouse Area	2,200	"The Fins," NW and N shears.
SL82-M	Upstream Left Abutment	2,200	NW and N shears.
SL82-N	Right Abutment, SL81-15 to SW-2	1,300	NW and N shears.
SL82-O	Right Abutment	1,100	"The Fins," and possible east-trending structures.
SL82-P	Upstream Left Abutment	1,100	"The Fins".
SL82-Q	Left Abutment, Downstream Portal Area	1,100	"Fingerbuster".
SL82-R	Left Abutment, Downstream Portal Area	1,100	"Fingerbuster".
	<b>TOTAL</b>	<b>26,600</b>	

\*Seismic lines listed in order of priority, seismic lines SL82-A through C are listed on Table.  
 \*\*See Figure 2.1.

TABLE 2.2: FY83 EXPLORATORY PROGRAM - PHASE II  
WATANA DAMSITE GEOPHYSICAL SURVEY

Seismic Refraction Line No.	Location	Approximate Length (ft)	Purpose
SL83-AA	Upstream Cofferdam Centerline	800	Depth of overburden and bedrock conditions.
SL83-BB	Upstream Cofferdam, River Centerline	1,100	Depth of overburden, bedrock conditions, and "The Fins".
SL83-CC	Main Dam Centerline	500	Depth of overburden and bedrock conditions.
SL83-DD	Downstream Cofferdam, River Centerline	1,100	Depth of overburden, bedrock conditions, and "Fingerbuster".
SL83-EE	Downstream Cofferdam Centerline	550	Depth of overburden, bedrock conditions, and "Fingerbuster".
SL83-FF	Downstream Cofferdam Centerline	550	Depth of overburden, bedrock conditions, and "Fingerbuster".
SL83-GG	Upstream Portal	550	Depth of overburden, bedrock conditions, and "The Fins".
SL83-HH	Main Dam River Centerline	3,850	Depth of overburden, bedrock conditions, and NW and N shears.
SL82-II	Main Dam Upstream Toe	550	Depth of overburden and bedrock conditions.
SL83-JJ	Main Dam Upstream of	550	Depth of overburden and bedrock conditions.
SL83-KK	Main Dam Downstream of Centerline	400	Depth of overburden and bedrock conditions.
SL83-LL	Main Dam Downstream Toe	550	Depth of overburden, bedrock conditions, and "Fingerbuster".
SL83-MM	Plunge Pool	500	Depth of overburden, bedrock conditions, and "Fingerbuster".
SL83-NN	Plunge Pool	1,100	Depth of overburden, bedrock conditions, and "Fingerbuster".
SL83-00	Plunge Pool	<u>2,200</u>	Depth of overburden, bedrock conditions, and "Fingerbuster".
	TOTAL	14,850	

TABLE 2.3: FY83 EXPLORATORY PROGRAM  
WATANA DAMSITE HAMMER DRILLING PROGRAM

<u>Drill Hole*</u> <u>Number</u>	<u>Location**</u>	<u>Approximate Depth (ft)***</u>	<u>Sampling</u>	<u>Purpose</u>
<u>Upstream Cofferdam</u>				
HD-AA	SL81-4	110	Alluvium and rock	Overburden material and thickness, bedrock surface, lithology and structure.
HD-BB	SL83-BB	120	Alluvium and rock	Same as above.
HD-CC	SL81-5	90	Alluvium	Overburden material and thickness.
HD-DD	SL83-AA and SL83-BB	100	Alluvium	Same as above.
<u>Main Dam</u>				
HD-EE	Centerline SL83-CC	85	Alluvium	Same as above.
HD-FF	Centerline SL83-CC and SL83-HH	130	Alluvium and rock	Overburden material and thickness; bedrock surface, lithology and structure.
HD-GG	Centerline SL83-CC	95	Alluvium	Overburden material and thickness.
<u>Downstream Cofferdam</u>				
HD-HH	SL83-DD	110	Alluvium and rock	Overburden material and thickness; bedrock surface, lithology, and structure.
HD-II	SL83-FF	95	Alluvium	Overburden material and thickness.
HD-JJ	SL83-FF	75	Alluvium and rock	Overburden material and thickness; bedrock surface, lithology, and structure.
HD-KK	SL83-EE and SL83-DD	80	Alluvium	Overburden material and thickness.
<u>Upstream Portals</u>				
HD-LL	SL83-GG	70	Alluvium	Same as above.
HD-MM	SL83-GG	70	Alluvium	Same as above.
HD-NN	SL83-BB	100	Alluvium	Same as above.
<u>Main Dam</u>				
HD-OO	Upstream Toe, SL83-HH and SL83-II	100	Alluvium	Same as above.
HD-PP	SL83-JJ and SL83-HH	130	Alluvium and rock	Overburden material and thickness; bedrock surface, lithology and structure.
HD-QQ	SL81-16	100	Alluvium	Overburden material and thickness.
HD-RR	Downstream Toe, SL83-HH and SL83-II	130	Alluvium and rock	Overburden material and thickness; bedrock surface, lithology and structure.

TABLE 2.3 (Cont'd)

<u>Drill Hole*</u> <u>Number</u>	<u>Location**</u>	<u>Approximate</u> <u>Depth (ft)***</u>	<u>Sampling</u>	<u>Purpose</u>
<u>Plunge Pool</u>				
HD-SS	SL83-MM	90	Alluvium	Overburden material and thickness.
HD-TT	SL83-MM and SL83-JO	90	Alluvium and rock	Overburden material and thickness; bedrock surface, lithology and structure.
	TOTAL	1,970		

\*Borings listed by priority.

\*\*See Figure 2.1

\*\*\*Assume 10 feet of rock drilling except 40 feet where rock will be sampled.



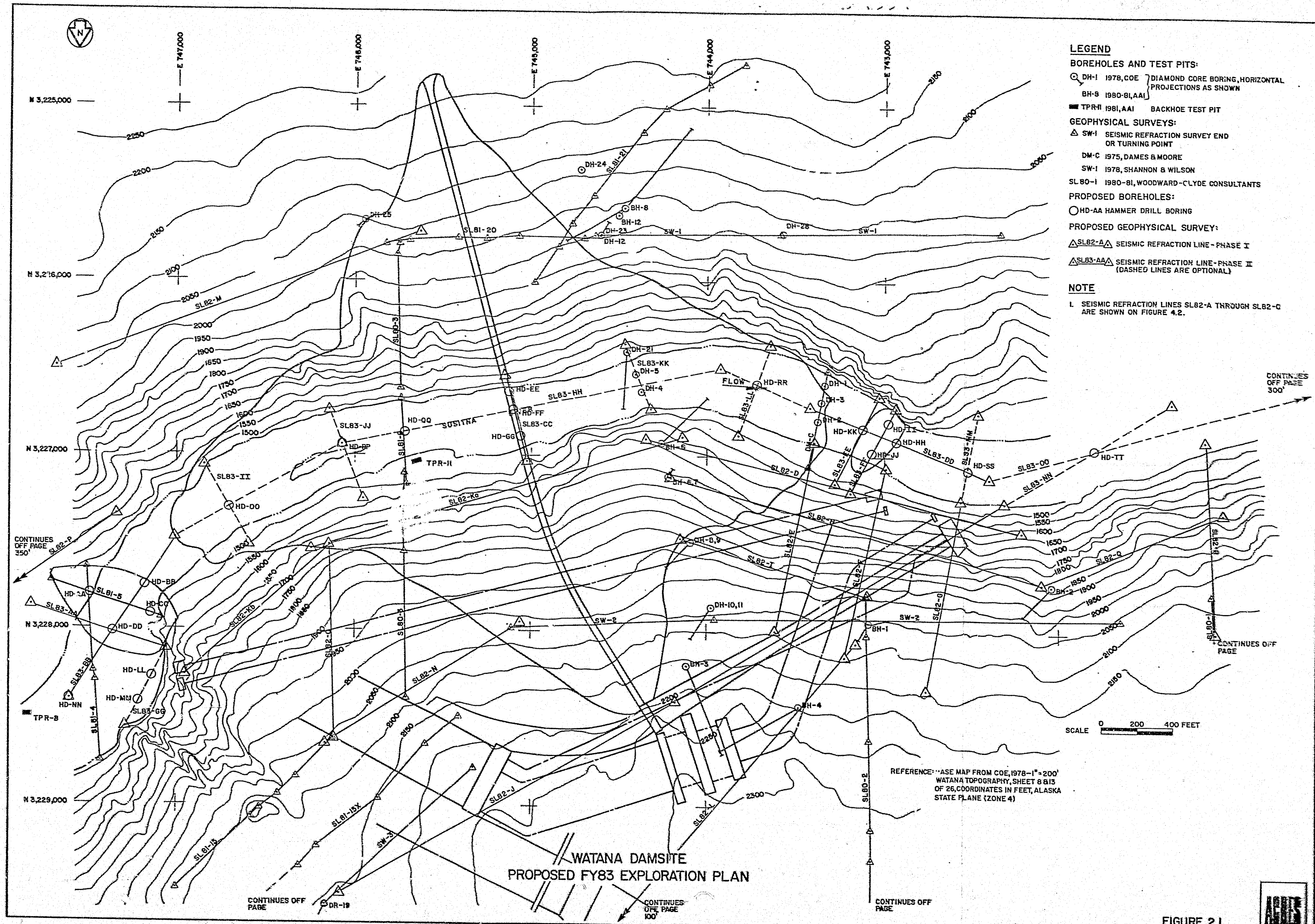


FIGURE 2.1



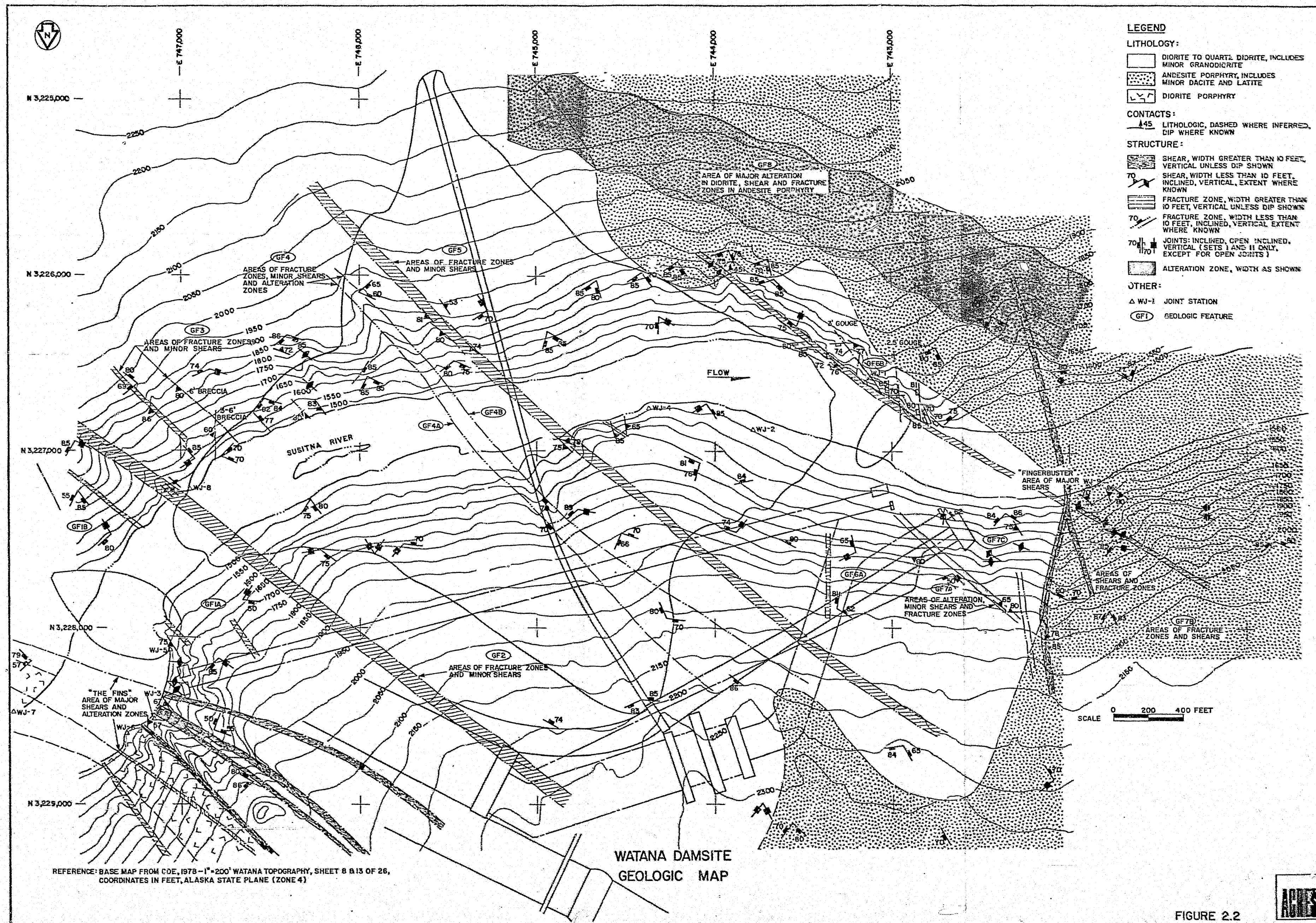


FIGURE 2.2



### 3 - BORROW SITE D

#### 3.1 - General

Borrow Site D has been identified as a zone ranging from 2,000-6,000 feet wide, stretching over 12,000 feet from a point on Deadman Creek to the approximate thalweg of the relict channel, including approximately 7,000 feet of exposure as bluffs overlooking the Susitna River, a total area of about 1,075 acres (Figure 3.1). The area has been identified as the most favorable source for impervious core material with a total estimated site requirement of approximately 8.25 mcy. The information, to date, on the borrow site is comprised of seismic refraction surveys, auger holes, several deep rotary drill holes, and shallow test pits.

#### 3.2 - Objectives

The objectives of the FY83 program are to obtain sufficient details to confirm the stratigraphy developed during feasibility, and develop a clear understanding of material properties, ground water, and permafrost conditions. Results of the FY83 will form the basis for the subsequent detailed design level investigations. The long-range objective of the Borrow Site D investigations will be to determine:

- (a) Borrow site stratigraphy and extent of each of the various identified units of interest to a level, adequate to confirmed volume-distance relationships, stripping parameters, and producible volumes.
- (b) Define site and adjoining areas, ground water, and permafrost conditions in order to allow optimization of production methodology, mining method, and water/frost handling.
- (c) Limits of desired excavation based on geologic conditions and desired material properties and determination of mining methods.
- (d) Continuance of material properties and their influence on production, processing, and placement requirements.

- (e) Engineering properties of the material, as required for placement in the fill, including placement water content, gradation, plasticity, and compaction characteristics. Explorations will place emphasis on detailing volumes of reserve with various properties.

### 3.3 - Approach

Since Borrow Site D lies within the relict channel areas, an exploration for borrow materials will also provide desired information on the relict channel stratigraphy.

The Phase I summer explorations on Borrow Site D will be limited to surficial outcrop mapping during early summer to delineate exposed borrow materials and bedrock. In addition, a limited seismic refraction survey will be performed in both the borrow area and relict channel, as well as a auger/rotary drilling program which is intended to provide high quality split spoon and "undisturbed" core samples to a depth of approximately 150 feet (Figure 3.2 and Table 3.1). The drilling will be performed with helicopter transportable rigs equipped with a variety of drill tools enabling it to utilize rotary with casing, hollow stem auger, and conventional wireline coring methods of advance. It is planned that each boring will commence with large diameter casing capable of obtaining 3-inch samples to an approximate depth of 75 feet. It is anticipated that from 75 to 150 feet the auger system will not be effective, thereby a wireline drill rod will be used with a "casing advancer" rotary tricone bit. Depending on drilling difficulty and sample recovery, it is likely that this method will, in a large portion, replace the augering method of advance. Normal advance procedure will involve use of frequent samples, as described in a following paragraph, with rotary drilling to overcome boulders and cobble zones and to advance between samples. Proposed borehole and seismic line locations are shown in Figure 3.2. The information collected during the summer program will be used to locate the drill sites for the larger diameter hammer drill boring to be conducted in the winter when land access is possible.

The winter Phase II program will be conducted as part of the relict channel exploration program and will involve use of a large hammer drill with large

inside diameter (6 inches) drill pipe (Figure 3.2 and Table 3.2). Continuous sampling of the blown out cuttings will provide large samples of the various stratigraphic units for use in geologic interpretation of the stratigraphy and in materials laboratory testing.

The drilling program in Borrow Site D will be directed to:

- Obtaining information in locations where the stratigraphy can best be delineated and sampled, and;
- Obtaining bulk samples of the potential borrow materials in locations where representative samples have not yet been obtained.

Due to the primary need for stratigraphic information and the fact that systematic pattern explorations of the borrow reserves will be conducted in the subsequent stage of design investigations, the emphasis of the FY83 Phase II program will be put on stratigraphic data collection. The sampling and instrumentation activities carried out in this program will be directed at maximizing the level of information obtained from each boring. Long-term instrumentation will be installed to provide baseline thermal and geohydrologic data. The various methods that will be utilized are described individually below:

- (a) Full depth sampling to provide material for geologic examination of the larger particle sizes and for laboratory testing.
- (b) Drive and core type sampling as is appropriate to include 2- and 3-inch standard split spoon with standard penetration blow counts. Denison sampling, and dry-blocked and conventional fluid circulation core barrel sampling.
- (c) Casing drive tests (open casing drive tests) for general correlation of stratigraphy and density.
- (d) "Undisturbed" samples, using such samples as fixed piston "Osterberg", Shelby, and other special split and tube type barrel samples.

- (e) Water level detection and monitoring both during and after drilling to detect various aquifers, perched water zones, and "dry" zones. Due to the preliminary indications that the area has a complex system of ground water levels, most borings will have a minimum of a standpipe type piezometer and those which penetrate a definite zone of interest may have pneumatic or electric piezometers installed. This system of observation points will be correlated with the relict channel installations.
- (f) Limited aquifer permeability testing such as falling head and/or constant head tests will be performed where possible to obtain order-of-magnitude information on the ranges of permeabilities of the materials in the borrow site.
- (g) Permafrost detection will be limited to observation of both seasonal and permanent frost during drilling, and installation of thermistors in many of the holes. The probes will then enable detection of permafrost in the following year after the frost has stabilized and of the depth of annual frost penetration during the winter. This information will then be utilized to evaluate the frost conditions likely to be encountered in excavation and at a later stage of design will be used with the results of all thermal investigations data to develop the borrow site production method to minimize the adverse effects of the frost in excavation and fill placement.

TABLE 3.1: FY83 EXPLORATORY PROGRAM - PHASE I  
WATANA RELICT CHANNEL/BORROW SITE D  
AUGER DRILLING PROGRAM

Boring* Number	Approximate Location***	Depth (ft)	Instrumentation	Purpose
AH-A	D/RC on SW-3	Assume all drill holes to 150 feet	SP	Expand stratigraphic correlations across D/RC.
AH-B	D/RC on DM-A	Same as above	P = 3 T	Same as above.
AH-C	D/RC at Inter- section of DM-A and DM-B	Same as above	P = 3 T	Same as above.
AH-D	D/RC, 100 feet E of SW - 5	Same as above	P = 3 T	Same as above.
AH-E	D/RC at inter- section of SW-3 and SL80-7	Same as above	SP	Same as above.
AH-F	RC on SL81-14	Same as above	P = 2	Same as above.
AH-G	RC on SL80-2	Same as above	P = 3	Same as above.
AH-H**	RC on SL80-1	Same as above	SP	Same as above.
AH-I**	RC on SL82-A	Same as above	SP	Same as above.
AH-J	RC on SL80-1	Same as above	P = 3	Same as above.
AH-K**	RC on SL82-B	Same as above	SP	Same as above.
AH-L	RC on SW-3	Same as above	SP	Same as above.
AH-M	D/RC on SL80-7	Same as above	P = 3 T	Same as above.
AH-N**	D/RC near TP-21	Same as above	SP	Same as above.
AH-O**	RC	Same as above	SP	Add detail to stratigraphy and material properties.
AH-P**	D	Same as above	SP	Same as above.
AH-Q	D	Same as above	P = 3 T	Same as above.
AH-R	D Near DR-14 on SW-3	Same as above	SP	Same as above.
AH-S**	D on SL80-8	Same as above	SP	Same as above.
AH-T**	RC at intersec- tion of SL81-14 and SL82-C	Same as above	SP	Same as above.
AH-U	RC on SL81-15X	Same as above	P = 3	Same as above.
AH-V	RC on SL82-A	Same as above	SP	Same as above.
AH-W	RC on SL80-2	Same as above	SP	Same as above.
AH-X	RC between DR-19 and DR-20	Same as above	T, SP	Same as above.

TABLE 3.1 (Cont'd)

Boring* Number	Approximate Location***	Depth (ft)	Instrumentation	Purpose
AH-Y**	D/RC on SL81-6	Assume all drill holes to 150 feet	SP	Add detail to stratigraphy and material properties.
AH-Z	D on SW-3	Same as above	SP	Same as above.
AH-AA	RC between DR-26 and DR-22	Same as above	P = 2, SP	Same as above.
AH-BB	RC on SL81-13	Same as above	P = 2, SP	Same as above.
AH-CC	RC on SL80-1	Same as above.	SP	Same as above.
AH-DD**	RC on SL82-B	Same as above.	SP	Same as above.
AH-EE	RC on DM-A	Same as above.	P = 1	Same as above.
AH-FF	D/RC near AH-D-8	Same as above.	P = 3	Same as above.
AH-GG	D on DM-A	Same as above.	SP	Same as above.
AH-HH	D near AN-D-10 and AN-D-11	Same as above.	SP	Same as above.
AH-II	D near AP-18	Same as above.	P = 1	Same as above.
AH-JJ	D between AP-10 and AP-11	Same as above.	SP	Same as above.
AH-KK	D near TP-14	Same as above.	P = 1	Same as above.
AH-LL	D near TP-10	Same as above.	SP	Same as above.
AH-MM	RC on SL81-14	Same as above.	P=1	Same as above.
AH-NN**	D/RC on DM-A	Same as above.	SP	Same as above.
AH-OO	D on DM-A	Same as above.	SP	Same as above.
AH-PP**	RC near DR-20	Same as above.	SP	Same as above.
AH-QQ	RC near SL81-15	Same as above.	SP	Same as above.
AH-RR	D near SL81-19	Same as above.	SP	Same as above.
AH-SS	D, SW of RR	Same as above.	SP	Same as above.
AH-TT	D, near TP-13	Same as above.	SP	Same as above.
AH-UU AH-VV AH-WW AH-XX	Miscellaneous, locations to be determined in field, also for more detailed investigation of freeboard dike.			

\*Borings listed by priority.

\*\*Auger borings adjacent to hammer drill holes.

\*\*\*See Figures 3.2 and 4.2.

D = Borrow Site D  
P = Piezometer tips (pneumatic)

RC = Watana Relict Channel  
SP = Standpipe Piezometer  
T = Thermistors

TABLE 3.2: FY83 EXPLORATORY PROGRAM - PHASE II  
WATANA RELICT CHANNEL/BORROW SITE D  
HAMMER DRILLING PROGRAM

Boring* Number	Approximate Location***	Approximate Depth (ft)	Instrumentation***	Purpose
HD-A	RC on SL80-1 near AH-H	200	P = 3 T	Stratigraphy material, properties, ground water, and permeability.
HD-B	RC on SL82-A near AH-I	100	P = 2 T	Same as above. On main thalweg.
HD-C	RC on SL82-B near AH-K	450	P = 5	Same as above.
HD-D	RC on SL82-B near AH-DD	300	P = 3	Same as above.
HD-E	RC near DR-20, and AH-PP	225	P = 3 T	Same as above.
HD-F	RC on SL80-2	50	P = 3	Same as above.
HD-G	RC on SW-3	150	P = 2	Same as above.
HD-H	D/RC on SL81-6 near AH-Y	425	P = 5	Same as above.
HD-I	RC near AH-O	425	P = 5 T	Same as above.
HD-J	RC on SL81-14 and SL82-C, near AH-T	525	P = 5 T	Same as above.
HD-K	D/RC on DM-A, near AH-NN	225	P = 3	Same as above.
HD-L	D/RC near TP-21 and AH-N	400	P = 5 T	Same as above.
HD-M	D on SL80-8, near AH-S	600	P = 3 T	Same as above.
HD-N	D on SW-4	250	P = 3	Same as above.
	TOTAL	4,325	P = 50 tips, allow 20 additional tips for miscellaneous locations.  T = 7 strings, allow 3 additional strings for miscellaneous locations.	

\*Borings listed by priority.

\*\*Auger borings adjacent to hammer drill holes.

\*\*\*See Figures 3.2 and 4.2.

D = Borrow Site D

P = Piezometer tips (pneumatic)

RC = Relict Channel

SP = Standpipe Piezometer

T = Thermistors



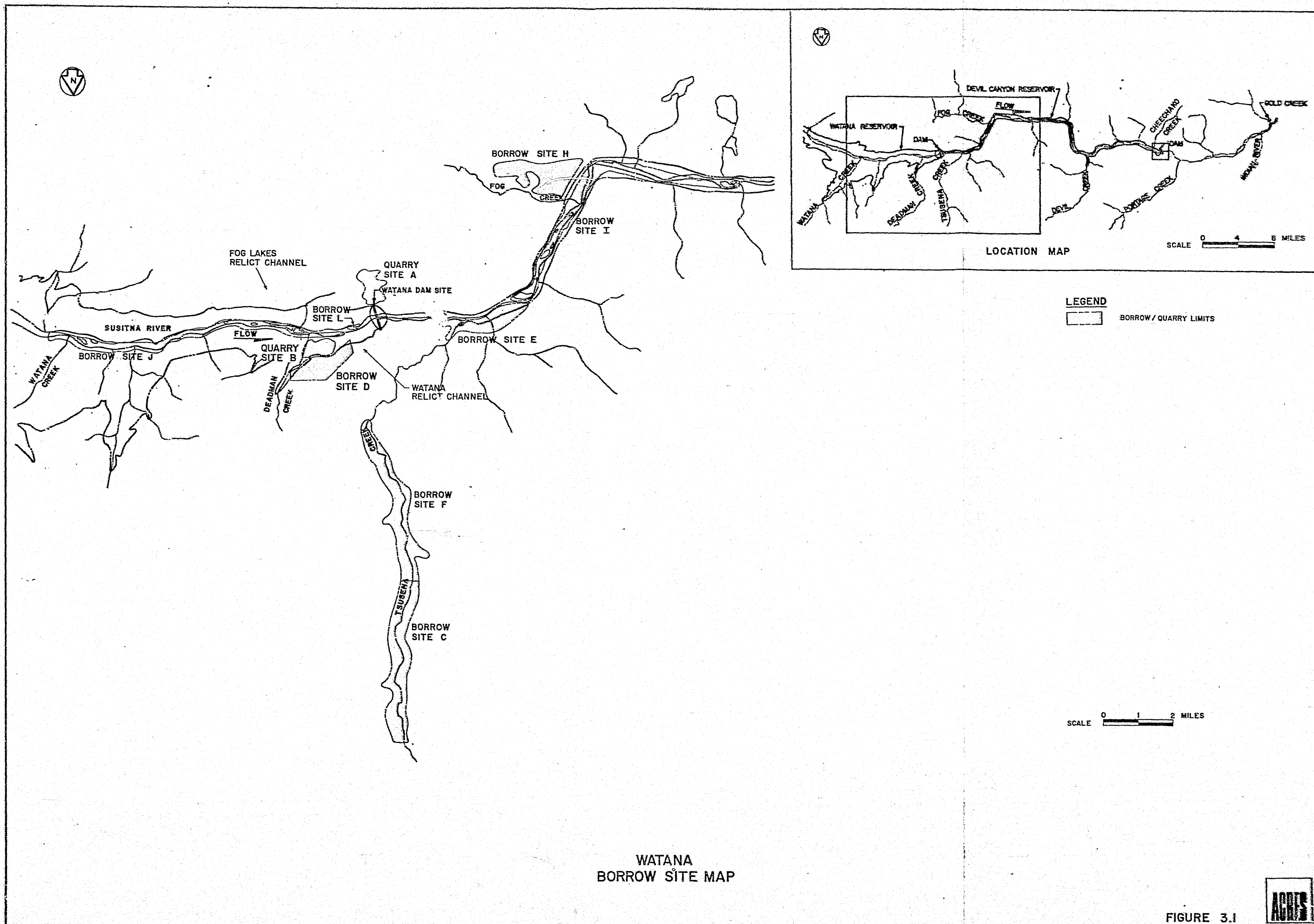


FIGURE 3.1





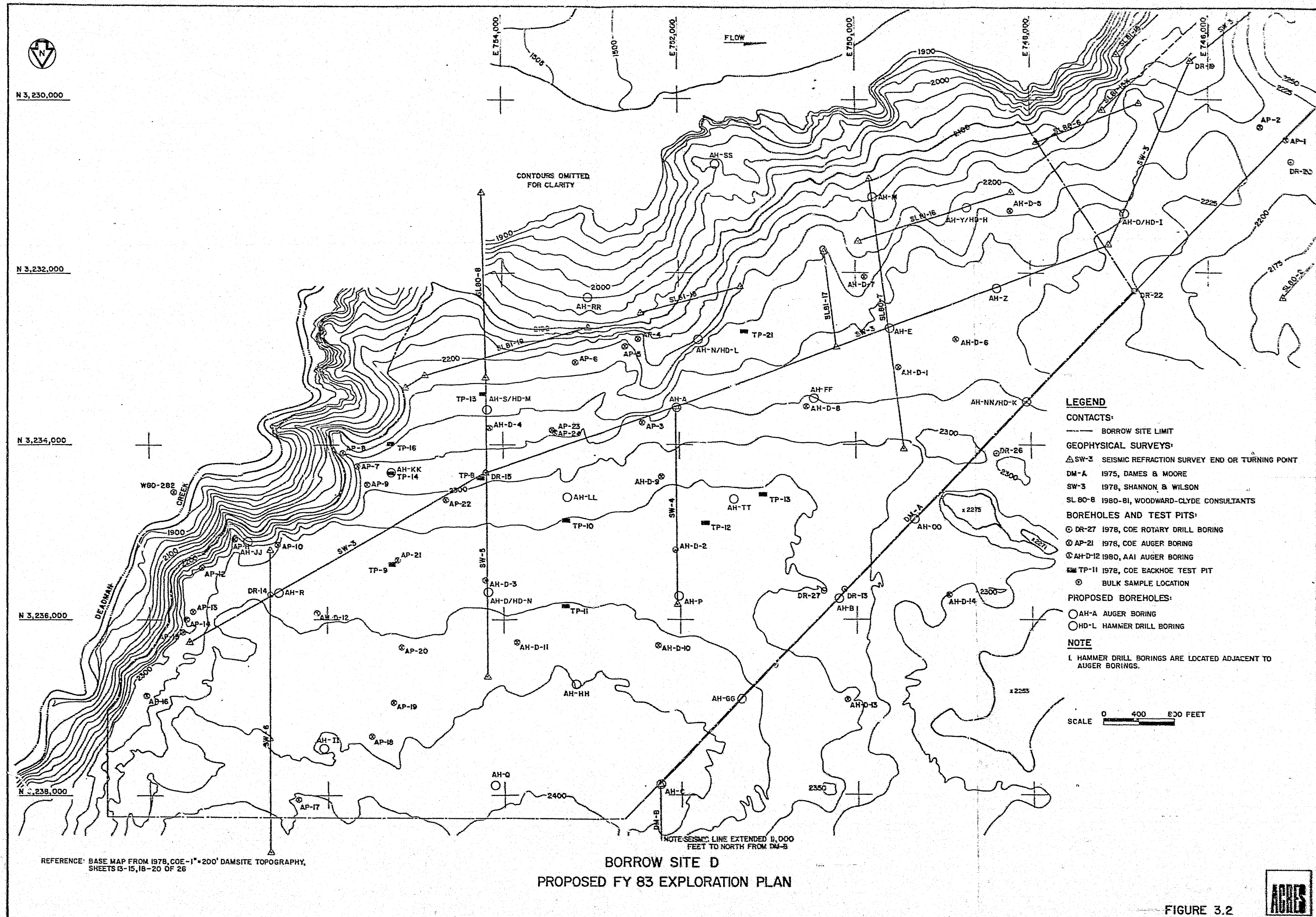


FIGURE 3.2



## 4 - RELICT CHANNEL

### 4.1 - General

Two areas have been identified on the banks of the proposed Watana reservoir where bedrock falls below the proposed reservoir elevation and, hence, provides a potential for reservoir leakage. The preliminary explorations and geologic mapping indicate that both of these areas may be abandoned Susitna River channels or "relict channels", one between Deadman and Tsusena Creeks, and the second in the Fog Lakes area<sup>(1)</sup>.

The concern regarding these areas are:

- (a) Potential for excessive reservoir leakage of such magnitude as to affect project economics.
- (b) Potential for excessive local gradients under reservoir head which might cause piping of material and, hence, induce progressive failure of the rim material with ultimate breaching of the reservoir.
- (c) Overburden instability or seismic liquefaction potential which could result in breaching of the reservoir confinement.
- (d) Crest settlement due to saturation and permafrost thawing.

### 4.2 - Objectives

The objective of the FY83 program will be to adequately assess the potential for these occurrences. The following factors must be adequately defined to allow proper evaluation of the physical behavior of the relict channels under project operating conditions.

- Stratigraphy;
- Material properties;
- Boundary conditions;

- Geohydrology;
- Permafrost conditions; and
- In-situ physical condition of materials.

#### 4.3 - Approach/Discussion

A two-phase program of exploration is proposed. The FY83 program will primarily address the more critical Deadman-Tsusena area relict channel which has been designated as "the Watana Relict Channel"<sup>(1)</sup>. Preliminary assessment at the Fog Lakes Channel show the freeboard at Fog Lakes is significantly higher than the Watana Relict Channel, and the local and overall gradient is much flatter and the flow path much longer than the Watana Relict Channel which minimizes any apparent hazard. It is therefore proposed to do only limited seismic refraction investigations in the Fog Lakes areas during FY83 with a more intense investigation being performed in the area during FY84 (Figure 4.1 and Table 4.1). That investigations will be conducted to assure that significant flow paths or liquefiable zones are not widespread in the Fog Lakes Relict Channel.

The Watana Relict Channel has a flow path of approximately 1-2 miles with an average gradient of about ten percent and a stratigraphy which (based on the information to date) does show potential for seepage. In addition, very limited subsurface data suggest that potentially liquefiable material may be present with the channel<sup>(1)</sup>.

The FY83 explorations will be jointly conducted with the Borrow Site D program since the borrow site lies within the confines of the relict channel areas (Section 3). The principal objective of the FY83 program is to obtain representative samples for stratigraphic identification, material properties distribution, and geohydrologic conditions in the relict channel.

The FY83 program for the relict channel will be performed in two parts, a summer - Phase I and winter - Phase II program. This is principally due to field logistics and the inability of transporting large drilling equipment overland

until the winter. Therefore, the summer program will use smaller helicopter transported drill rigs that will evaluate the upper 100-150 feet of the relict channel. These rigs will be the same rigs used in the Borrow Area D investigation (Section 3). Data from this program will be used in detailing the winter program. The objectives of FY83 are to:

#### Phase I

The objectives of Phase I are to:

- Perform surfacial mapping of the relict channel;
- Define the surface and subsurface hydrologic regime in the upper 100-150 feet;
- Obtain undisturbed and disturbed samples at depth to identify stratigraphy and material properties;
- Install instrumentation for monitoring ground water and permafrost; and;
- Delineate extent of Fog Lakes Relict Channel.

#### Phase II

The objectives of Phase II are to:

- Confirm the geometry of the relict channel;
- Obtain large bulk and undisturbed sample at depth for stratigraphic delineation and material properties;
- Evaluate the geohydrologic regime where possible; and
- Install instrumentation for monitoring ground water and permafrost.

Phase I will involve drilling approximately 50 borings distributed throughout the Watana Relict Channel and limited geophysical program (Table 4.2). Several of these borings will also serve for investigating Borrow Site D. Proposed boring locations are shown in Figures 4.2 and 4.3.

The drill equipment will have capability of drilling a 6-inch sampler in the upper 20-30 feet; reducing to 4-inch samples to 75 feet, and subsequently reducing to standard 2-inch samples to full depth of 150 feet. The intent is to

obtain a large diameter sample as possible to maximum depth with the intent of determining the following conditions

- Soil moisture content;
- Geohydrologic condition;
- Soil density;
- Soil gradation;
- Stratigraphy and sedimentary origin;
- Permafrost conditions; and
- Obtaining samples for laboratory testing to determine plasticity limits, compaction, permeability and dispersion.

The results of the summer investigations are expected to provide a model of stratigraphy, ground water and permafrost regime, and material properties in the upper 100-150 feet of the relict channel for use in subsequent design and estimates, and to assist in laying out the deep drilling program to be performed in the winter program.

Phase II will utilize the same large diameter hammer drill that was described in the Borrow Site D (Section 3). Approximately 15 holes are planned to penetrate to bedrock in the relict channel. The drill cuttings will be used in stratigraphic delineation and borrow materials testing. In addition, the drive and drilled sampling methods utilized in the Phase II will be used to obtain "undisturbed" samples for precise laboratory tests. Proposed boring location for the hammer drill are shown in Figures 4.3 through 4.5.

As in Borrow Site D, emphasis in the sampling program will be placed on obtaining maximum size samples for stratigraphic information and materials investigations. The various sampling methods that will be available are listed below.

- (a) Full depth sampling and logging of drill cutting which will provide limited supplemental material and stratigraphic information.
- (b) Drive and coring sampling, using 2- and 3-inch standard split spoon samples, as well as, special samplers, such as the Denison.

- (c) "Undisturbed" sampling, using special "floating tube" or piston type samples such as the "Gus" and "Osterberg" and various diameters of Shelby and similar split or solid thin wall sampling.
- (d) Casing drive testing, using the hammer drill (winter) to ascertain relative variations in density.
- (e) Water level detection and monitoring, both during and after to drilling operations.
- (f) Installation of well casing screens and piezometers for subsequent observations and aquifer testing.
- (g) Permafrost monitoring through cutting temperature measurement, observation of ice in the samples taken, and installation of frost probe PVC pipe for continued observation. If significant frost is detected at depth, a full thermistor string may be installed if thought to be appropriate.



TABLE 4.1: FY83 EXPLORATORY PROGRAM  
FOG LAKES RELICT CHANNEL  
GEOPHYSICAL SURVEY

Seismic Refraction Line No.*	Location**	Approximate Length (ft)	Purpose
SL82-FL-A	Channel A	22,000	Determine gradient of channel.
SL82-FL-B	Channel B	15,400	Determine bedrock gradient of channel.
SL82-FL-C	Channel A and B	22,000	Width of Channel A and B.
SL82-FL-D	Channel A	22,000	Determine bedrock gradient of possible additional channels.
SL82-FL-E	Channel A	15,400	Upstream width of channel.
SL82-FL-F	Channel B	9,900	Width of channel.
SL82-FL-G	Channel B	5,500	Width of channel.

\*The seismic lines shown here are based on limited geologic data suggested for more complete definition of the channels. The number and orientations of seismic lines will be based on additional geologic mapping.

\*\*See Figure 4.1

TABLE 4.2: FY83 EXPLORATORY PROGRAM - PHASE I  
WATANA RELICT CHANNEL GEOPHYSICAL SURVEY

Seismic Refraction Line No.*	Location**	Approximate Length (ft)	Purpose
SL82-A	Relict channel	6,600	Bedrock depth, overburden conditions.
SL82-B	Relict channel	6,600	Bedrock depth, overburden conditions.
SL82-C	Relict channel	1,100	Tie in lines SL81-14 and SL80-2 to complete coverage.
SL82-S	Relict channel	6,600	Detail of main relict channel thalweg.
Or			
SL82-T	Relict channel	6,600	Same as above.
SL82-U	Relict channel	1,500	Tie in lines SL81-16 and SL81-15X to complete coverage.
SL82-V	Borrow Site D and relict channel	1,100	Tie in lines SL81-16 and SL81-18 to complete coverage.
SL82-W	Relict channel	3,500	From SL81-13 to Tsusena Creek to complete coverage.

\*Seismic lines listed in order of priority, SL82-D through SL82-R on Table \_\_\_\_.  
\*\*See Figure 4.2.



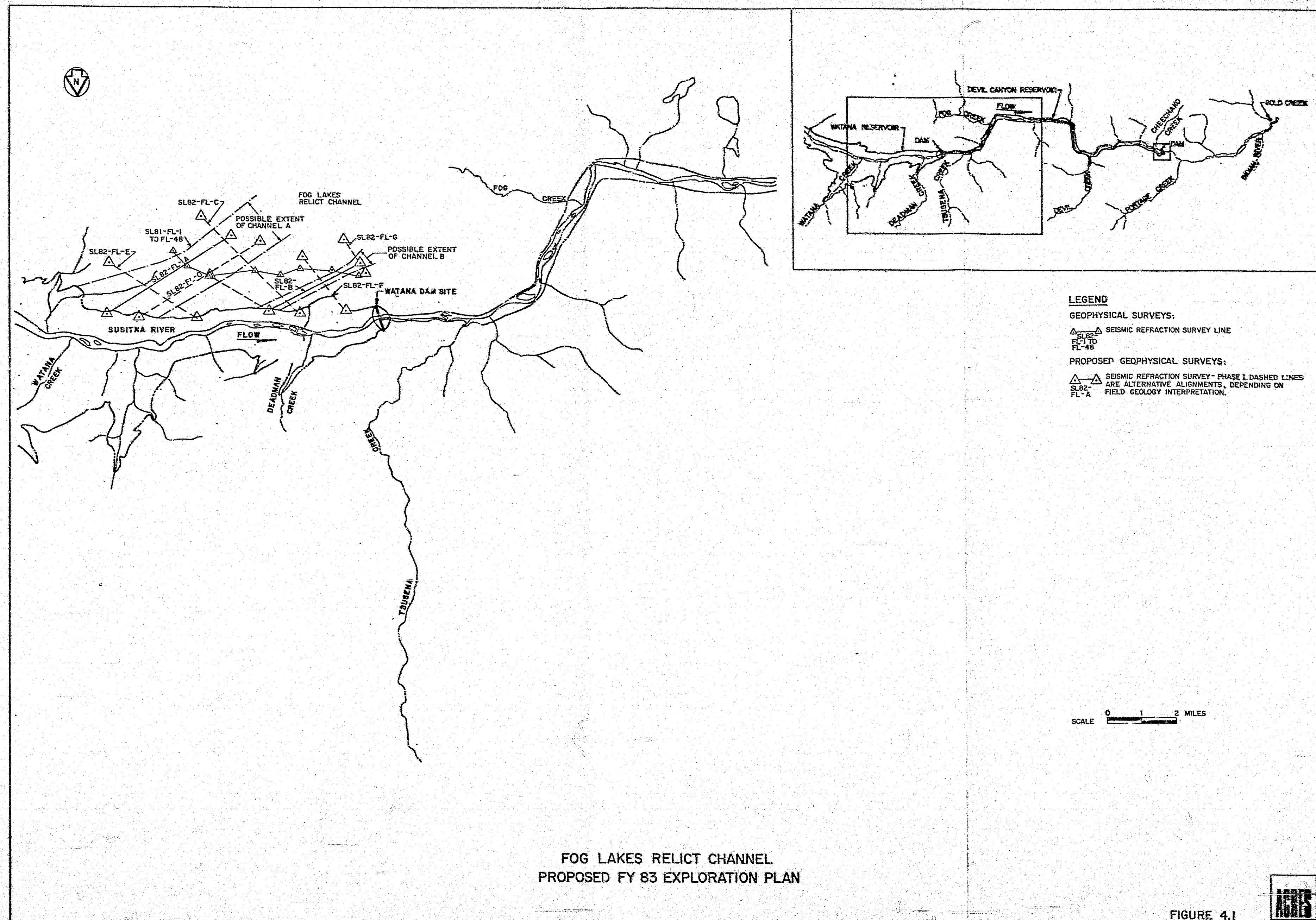
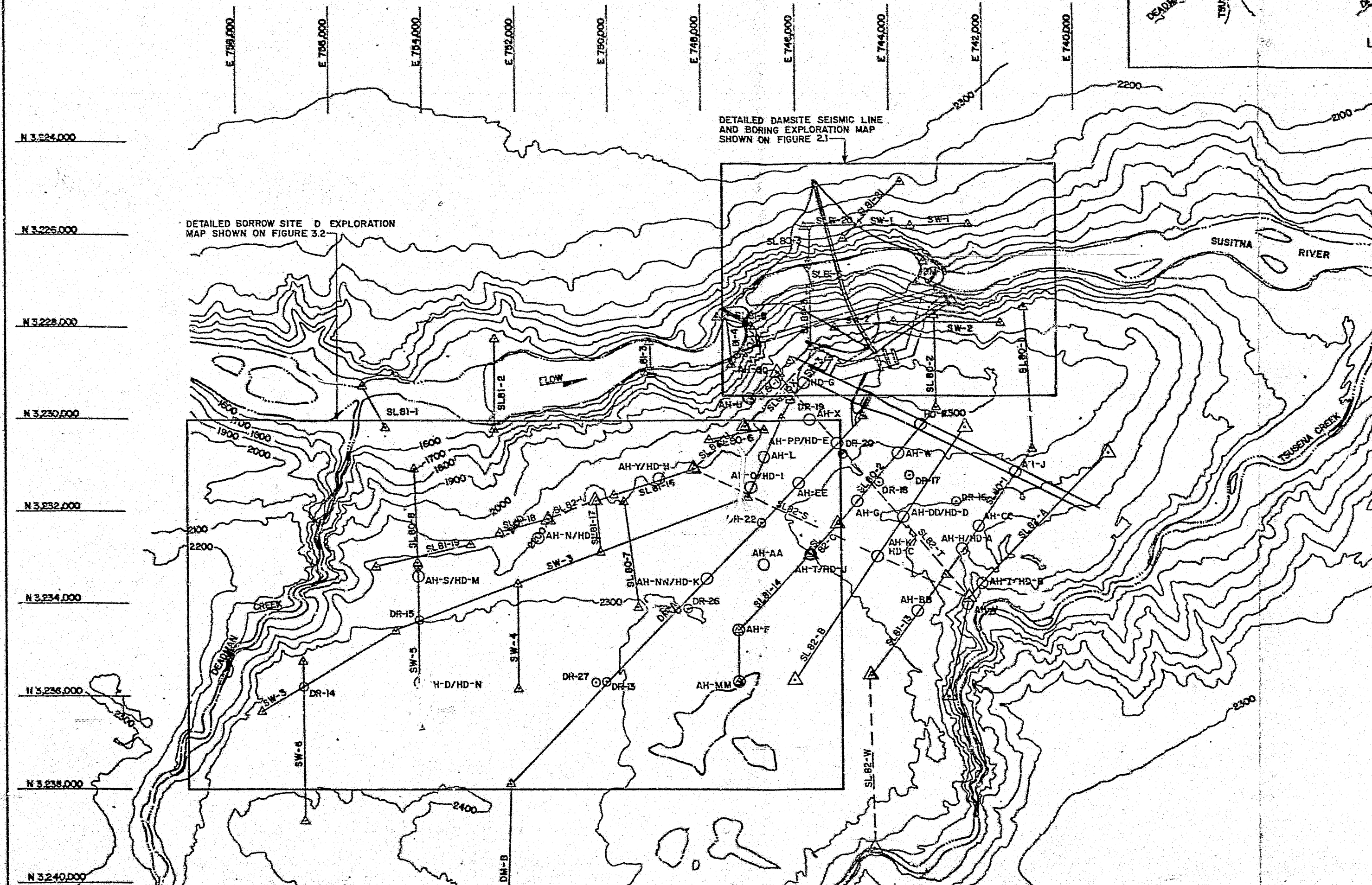
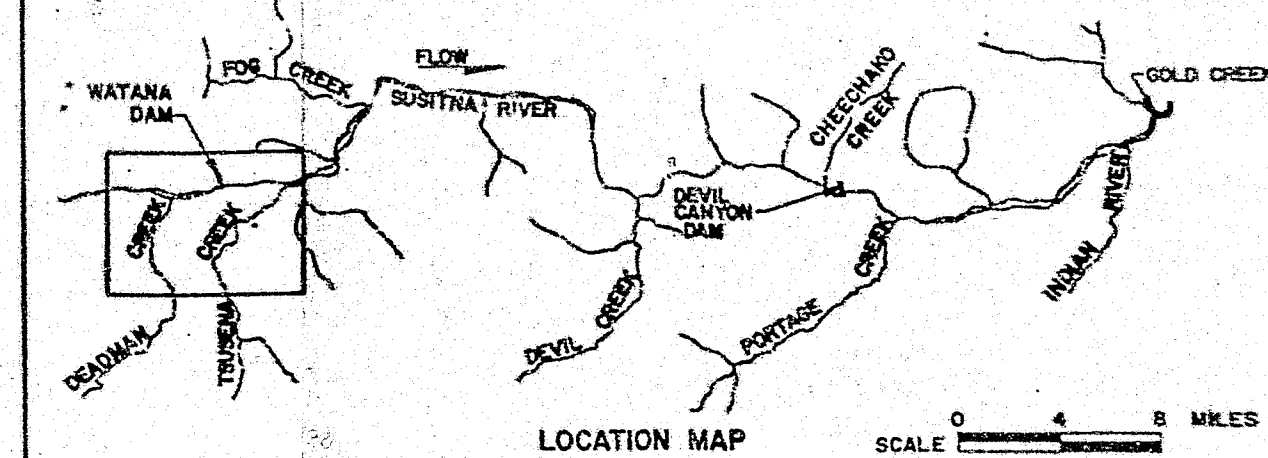
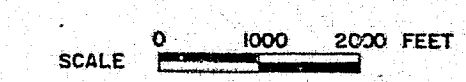


FIGURE 4.1





- LEGEND**
- BOREHOLES AND TEST PITS:**
- DR-15 1978, COE ROTARY DRILL BORING
- GEOPHYSICAL SURVEYS:**
- △ SEISMIC REFRACTION SURVEY END OR TURNING POINT
  - DM-C 1975, DAMES & MOORE
  - SW-2 1978, SHANNON & WILSON
  - SL81-5 1980-81, WOODWARD-CLYDE CONSULTANTS
- PROPOSED BOREHOLES:**
- AH-G AUGER BORINGS
  - HD-F HAMMER DRILL BORING
- PROPOSED GEOPHYSICAL SURVEYS:**
- △ SL △ SEISMIC REFRACTION LINES, DASHED LINES ARE OPTIONAL.
- NOTES**
1. HAMMER DRILL BORINGS, EXCEPT HD-F AND HD-G, ARE LOCATED ADJACENT TO AUGER BORINGS.
  2. SEISMIC REFRACTION LINES SL82-D THROUGH SL82-R ON FIGURE 2.1.



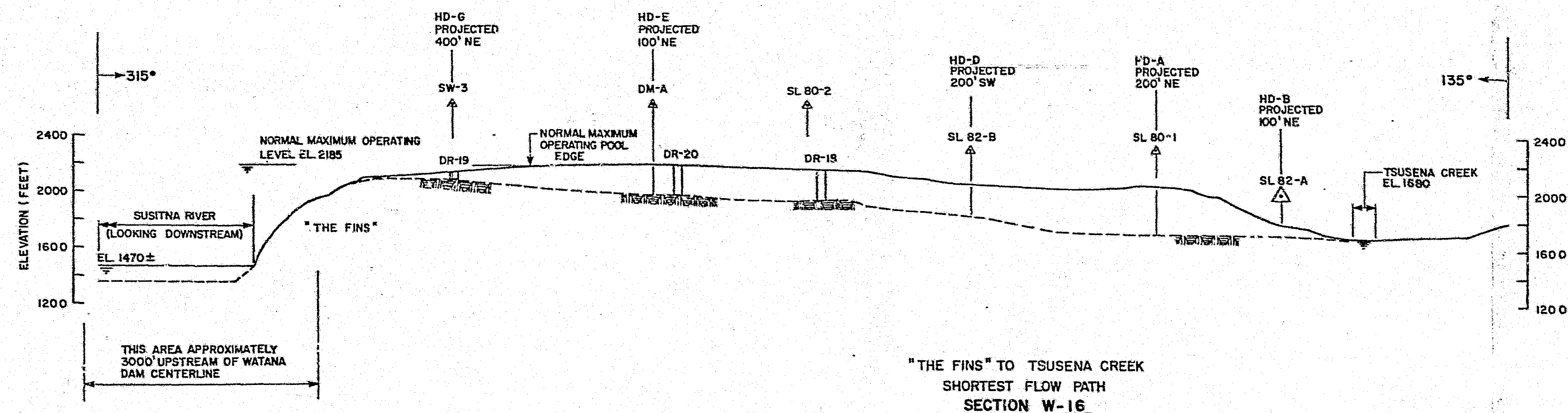
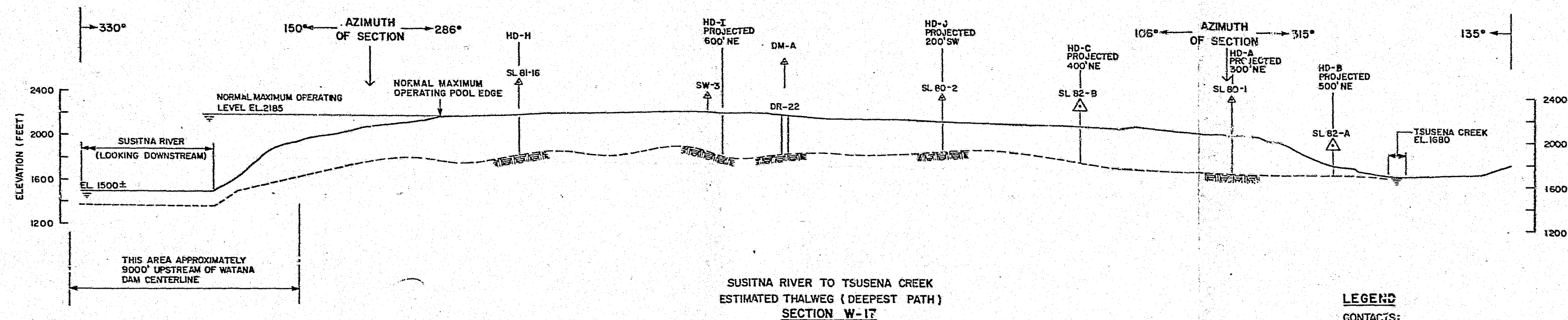
REFERENCE: 1"=200' COE, 1978 WATANAH DAMSITE TOPOGRAPHY, SHEET 1 THROUGH 26

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**WATANAH RELICT CHANNEL  
PROPOSED FY 83 EXPLORATION PLAN**

FIGURE 4.2





# **LEGEND**

## **CONTACTS:**

--- APPROXIMATE TOP OF ROCK

## **GEOPHYSICAL SURVEYS:**

▲ SW-1 INTERSECTION WITH SEISMIC REFRACTION LINE

DM-C 1975, DAMES & MOORE

SW-1 1978, SHANNON & WILSON

SL 80-1 1980, WOODWARD-CLYDE CONSULTANTS

SL 81-2 1981, WOODWARD-CLYDE CONSULTANTS

## **BOREHOLES:**

DR-19 COE ROTARY DRILL BORING

## **PROPOSED BORE HOLES:**

HD-A HAMMER DRILL BORING

## **PROPOSED GEOPHYSICAL SURVEY:**

▲ SL 82-A SEISMIC REFRACTION LINE

## **NOTES**

1. PROFILE AND SEISMIC LINE LOCATIONS SHOWN ON FIGURE 4.4.
2. SECTION ALONG DM-A SHOWN ON FIGURE 4.5.
3. VERTICAL AND HORIZONTAL SCALE EQUAL.
4. SURFACE PROFILE FROM 1" = 200' TOPOGRAPHY, COE 1978 TOPOGRAPHY GENERALIZED TO ±25 FEET.
5. TOP OF ROCK NORTHWEST OF SL 80-1 IS PROJECTED UP TO 300 FEET TO PORTRAY ACTUAL THALWEG PROFILE.
6. AUGER BORINGS IN RELICT CHANNEL ARE SHOWN ON FIGURE 4.2.

SCALE 0 400 800 FEET

**WATANA RELICT CHANNEL PROFILES**





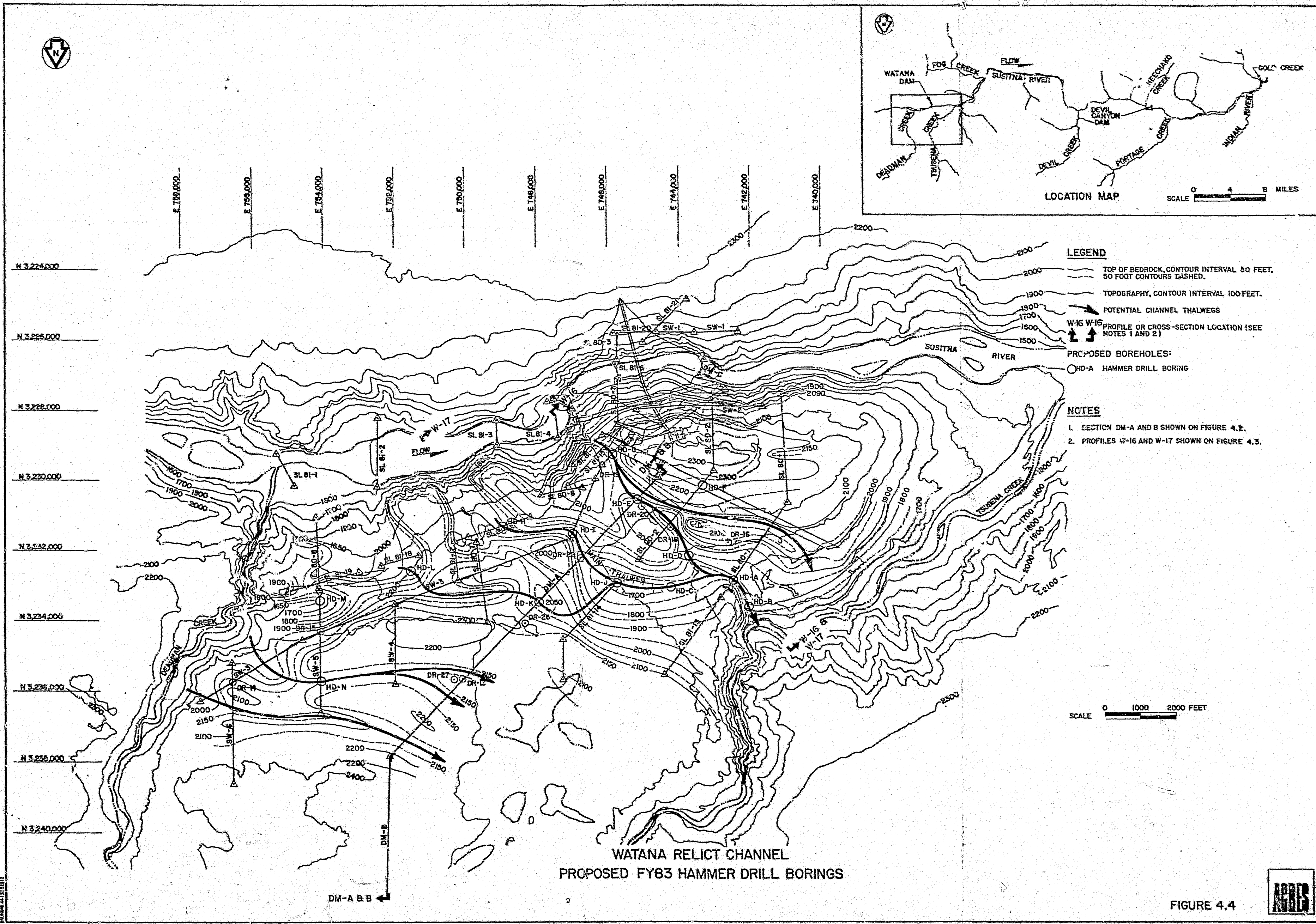
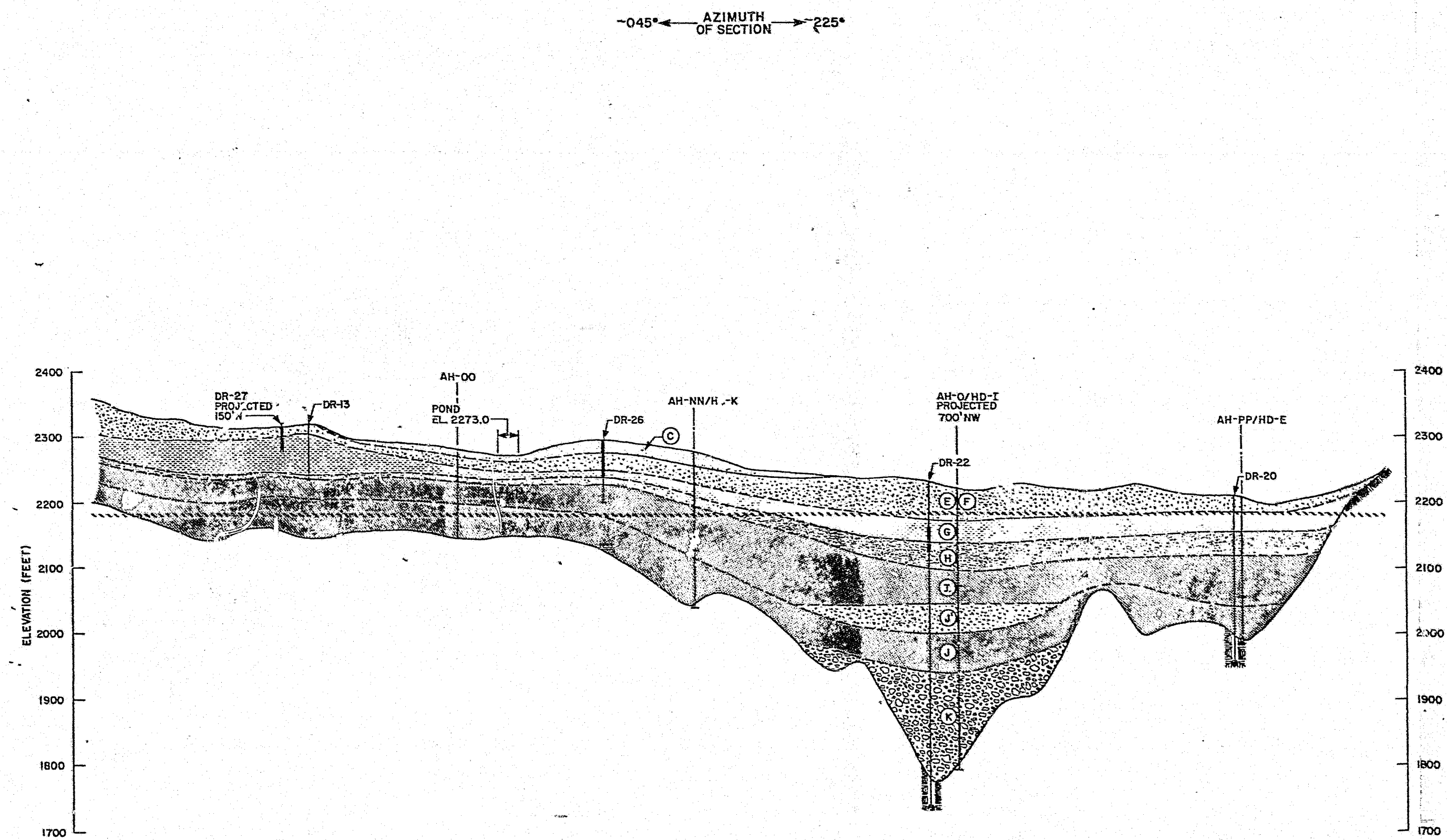


FIGURE 4.4

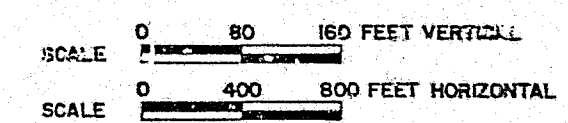




- LEGEND**
- LITHOLOGY:**
- |  |      |  |   |
|--|------|--|---|
|  | C    |  | I |
|  | E, F |  | J |
|  | G    |  | K |
|  | H    |  |   |
- CONTACTS:**
- KNOWN UNIT CONTACT
- - - INFERRED UNIT BOUNDARY
- BEDROCK SURFACE WHERE DRILLED.
- NORMAL MAXIMUM OPERATING LEVEL EL. 2185
- BOREHOLES:**
- DR-26 COE ROTARY CORE BORINGS
- NOTED AS FROZEN DURING DRILLING
- PROPOSED BOREHOLES:**
- HD-E HAMMER DRILL BORING
- AH-B AUGER BORING

**NOTE**

1. LOCATION OF SECTION ALONG SEISMIC LINE DM-8, ON FIGURE 4.4.



WATANA RELICT CHANNEL-EXPANDED THALWEG SECTION



## 5 - BORROW SITES E and I

### 5.1 - General

Borrow Sites E and I include the Tsusena Creek outwash plain and Susitna River flood plain from a point approximately 2-1/2 miles downstream of the Watana dam site to a maximum distance of approximately 12 miles downstream (Figure 3.1). This material has been identified as the most favorable source for concrete aggregate, filter sand and gravel, and dam shell gravel requirements totaling approximately 40 million cubic yards<sup>(1)</sup>. The information in these areas, to date, has been compiled from seismic refraction surveys, air photo interpretation, and various test pits and shallow auger holes.

### 5.2 - Objectives

The overall objective of the Borrow Site E and I investigation is to obtain four types of information needed for refinement of construction processing and production costs, confirmation of haul distances to obtain adequate reserves and development of engineering design parameters for dam design. These are:

- (a) Borrow site configuration, including definition of confirmed volume-distance relationships, lateral and vertical extent of suitable materials, and basic groundwater level and stripping depth parameters necessary to allow optimization of production methodology, selection of least-cost mining methods, and determination of in-place reserve figures.
- (b) Limits of reasonable excavation depth determination based on geologic constraints, stratification of deposit, and inherent stability of the materials. These limitations will have significant impact on the actual economically recoverable reserves which, in turn, can be expected to influence maximum excavation depth, net excavation losses, and, consequently, equipment selection and overall land requirements for adequate development.

- (c) Material gradations, including both typical and local variations in the material quality, which will influence production methods and requirements and possible processing.
- (d) Engineering properties of the material for use in concrete, filters, and dam shells for use in selection of processing requirements and optimization of placement utilization as determined by cost.

The objectives of the FY83 program are to:

- (a) Confirm the accuracy of previous seismic data and, therefore borrow site quantity and reserve calculations; and
- (b) Determine material properties.

### 5.3 Approach/Discussion

The amount of work that can be done within the FY83 is limited by time and the sampling equipment which will be mobilized to the site. The large hammer drill, which is planned for use in the Relict Channel and river areas (Sections 2 through 4), will be utilized to drill several confirmatory borings to maximum practical excavation depth below river level in the primary source area of Borrow Site E with the intent of verifying the seismic data and obtaining sample gradations and suitable test samples (Figure 5.1 and Table 5.1). Use of this piece of equipment should verify the general overall suitability of the source, ascertain the approximate upper size limits and range of the gradation, provide adequate samples for physical and durability suitability, and give an indication of variability of the deposit with depth.

Subsequent studies for final design investigations will determine the actual mineable limits, extent of variation in materials, ground water elevation, and the processing that will be required for each of the major gradation variations.

The FY83 summer program in Borrow Sites E and I will be limited to completion of surficial geologic mapping during Phase I to delineate all exposures of rock and



borrow materials with the intent of defining material limits and geomorphologic features which may give indications of expected material gradations. It is anticipated that the information obtained from the mapping will be limited to identification of relict terraces, alluvial fan deposits, and river flood plain limits within the borrow site. Phase II borehole locations will also be established during the summer program.

The winter program will involve use of the large hammer drill with large inside diameter pipe to continuously sample the site from surface to maximum practical excavation depth which is estimated at 125 feet below water level. The drilling will be distributed over the length of Borrow Site E to provide a general overall view of the borrow materials. Due to the wide spacing of these borings, stratigraphic correlation is not expected to result from this drilling. The borings will be concentrated along the margins of the site to meet the criteria below:

- (a) Ready access and relatively level drill setup to minimize setup time and, thereby, maximize the number of holes drilled.
- (b) Placement of at least ten of the holes on previous seismic lines to verify the seismic data interpretation.
- (c) Placement of several holes along the active river margin to verify the off-end-of-line seismic interpretation of alluvial depth and composition.
- (d) Placement of a line of holes near the north limit of the flood plain to confirm the average depth of alluvium and to assure that bedrock level does not rise rapidly in the north and east portions of the borrow site.
- (e) Spacing of the borings so as to optimize the interpretative value of the sampling, both for determination of the distribution of gradations and for the stratigraphic conditions that may vary from area to area in the borrow site.

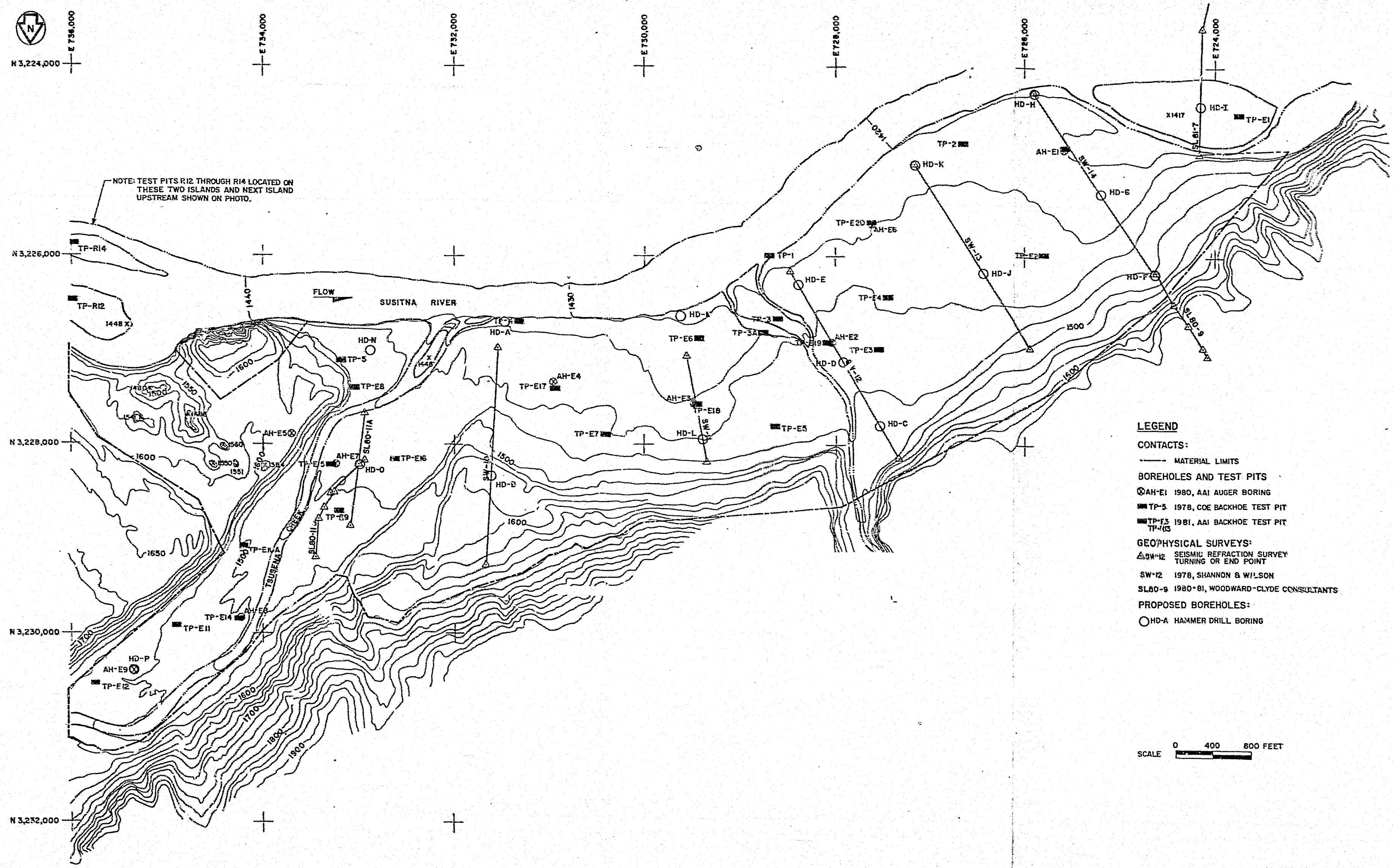
Borehole sampling will be conducted as listed below:

- (a) Primary sampling will be full depth cuttings sampling up to 4 inch size particles.
- (b) Split spoon or Denison/Core Barrel sampling, as necessary, to sample fine sand, silt, or clay layers which may be encountered.
- (c) Casing dive tests (open casing drive tests) for general correlation of stratigraphy versus density for use in estimating excavation requirements. Since the borrow site is planned for dragline excavation, detailed density testing is not considered appropriate.
- (d) Water table detection and monitoring to provide information on potential dry versus wet excavation. Because the water table is expected to range from 10-30 feet in depth, simple standpipe piezometric monitoring will be utilized to allow continued monitoring of seasonal variations of the water table with rainfall and river level.
- (e) Frost detection will be limited to direct observation of drilling rate and temperature measurements of cuttings. No permafrost is expected in the borrow site, but the depth of seasonal frost will be measured and checks will be made throughout drilling operations to record any indications of possible permafrost. If any permafrost is detected, thermal probe standpipes will be installed in the appropriate zones to detect the extent of the affected zone. In the remaining holes, standpipes will be installed to approximately 30 feet to provide the necessary anchorage length and reading coverage for detection of annual frost penetration thermal probe headings.

TABLE 5.1: FY83 EXPLORATORY PROGRAM - PHASE II  
BORROW SITE E HAMMER DRILLING PROGRAM

Boring* Number	Approximate Location**	Approximate Depth (ft)	Instrumentation***		Purpose
HD-A	SW-10, south end	60			Define stratigraphy, check on seismic line picks, material sampling.
HD-B	SW-10, north end	125	SP	FP	Same as above.
HD-C	SW-12, north end	110	SP	FP	Same as above.
HD-D	SW-12, center	75	SP		Same as above.
HD-E	SW-12, south end	125		FP	Same as above.
HD-F	SL80-9, north end	125	SP	FP	Same as above.
HD-G	SL80-9, center	110	SP	FP	Same as above.
HD-H	SL80-9, south end	120		FP	Same as above.
HD-I	SL81-17	75			Same as above.
HD-J	SW-13, center	75	SP	FP	Same as above.
HD-K	SW-13, south end	175			Same as above.
HD-L	SW-13, north end	60	SP	FP	Same as above.
HD-M	On river bank	90			Same as above.
HD-N	On river bank upstream of Tsusena Creek	70			Same as above.
HD-O	At intersection of SL80-11A & SL80-11B	60	SP		Same as above.
HD-P	Upstream on Tsusena Creek	100	SP		Same as above.
	TOTAL	1,555	SP = 9 installations, allow 1 miscellaneous installation.		
			FP = 8 installations, allow 2 miscellaneous installations.		

\*Borings listed by priority.  
 \*\*See Figure 5.1.  
 \*\*\*SP = Standpipe piezometer.  
 FP = Frost probe.



REFERENCES: BASE MAP FROM CGE, 1978-1"=200' WATANA TOPOGRAPHY, SHEET 6 & 11 OF 26  
 R&M, 1981-1"=400' DEVIL CANYON RESERVOIR MAPPING, FLIGHT  
 5 (6-8), MANUSCRIPT 2  
 COORDINATES IN FEET, ALASKA STATE PLANE (ZONE 4)

# BORROW SITE E PROPOSED FY83 EXPLORATION PLAN

FIGURE 5.1



## 6 - LABORATORY TESTING

### (a) General

The material testing program for FY83 will be designed for providing ready assistance in data interpretation. To expedite the testing program, a field laboratory will be established for performing routine soil tests to include:

- Gradation;
- Hydrometer;
- Moisture;
- Atterberg limits; and
- Proctor.

The more sophisticated tests will be performed by outside laboratories as required.

### (b) Damsite

As discussed in Section 2, petrographic analyses will be performed on specimens of rock and shear material obtained during geologic mapping activities. While petrologic rock type identification in slab section will suffice for delineation of the different rock types, full thin section petrographic analysis may be required for samples taken in contact or shear zones. The greater detail of the thin section identification can provide valuable data concerning the emplacement sequence and subsequent shearing and vein injection history, which will be of significance in evaluation of anticipated alteration and rock strength at depth in the pluton. The samples will be taken from outcrops and from core drilling to date, with an estimated 30 identifications and descriptions being performed.

During Phase II, approximately 20 holes averaging 100-feet deep (60 feet of overburden) are planned using a hammer drill. Limited spoon and barrel

sampling will be performed, but a majority of the sampling will be retrieval of air-educted alluvium through use of a cyclone separator. The estimated number of samples and routine tests to be performed on these samples are shown in Table 7.1

More detailed tests to be performed on undisturbed and other material samples will be undertaken following Phase II activities when an understanding of the subsurface materials in the damsite has been determined.

(c) Relict Channels/Borrow Site D

The material properties in the relict channel areas are of significance both to the geologic understanding of the areas and to evaluation of the long-term stability of the area. The Fog Lakes Relict Channel geologic mapping may produce a limited number of bagged samples of alluvial deposits from face exposures, which will be submitted to standard sieve and hand specimen descriptive analyses for general identification purposes. No detailed material testing is planned for the FY83 period in the Fog Lakes Relict Channel.

The material testing program in the Watana Relict Channel will be conducted in conjunction with the Borrow Site D testing. The principal objective of the Phase I work is to identify and define the type and extent of stratigraphic units in the Borrow Site D and the relict channel. Therefore, the soils testing program during Phase I will be limited to routine tests as shown in Table 7.1. Undisturbed and samples taken during this phase will be stored until data interpretation has advanced to the level where a sophisticated material testing program can be planned. However, the tests that may likely be undertaken for the relict channel and Borrow Site D are:

- Relict Channel

- . Permeability;
- . Consolidation;



- . Dispersion;
- . Strength/density testing; and
- . "In-situ" triaxial strength.

- Borrow Site D

- . Remolded permeability;
- . Dispersion;
- . Triaxial strength on remolded material;
- . Dilation (dynamic);
- . Dynamic shear strength;
- . Freez-thaw; and
- . Thermal properties.

It is expected that the detailed testing program will be prepared after Phase I activities.

(d) Borrow Site E

Because this area is intended for use as concrete aggregate, filter, and shall material, a number of tests may be run after the completion of the winter program, as shown on Figure 7.1. Because the material is scheduled for processing, in-situ properties such as moisture, strength, and density will not be sampled to a significant extent. However, extensive samples obtained during the drilling program will be run for gradations. A few representative hydrometers will be performed to provide typical data on settling times and water quality effects of the fines in washing and processing operations. Other sophisticated laboratory testing that may be run on composite samples are:

- Sulphate-soundness;
- L.A. abrasion;
- Freeze-thaw;
- Soundness (wetting-drying);
- Proctor compaction;
- Density-strength testing; and
- Bulk permeability after washing.



## 7 - RESULTS OF PHASE I INVESTIGATION

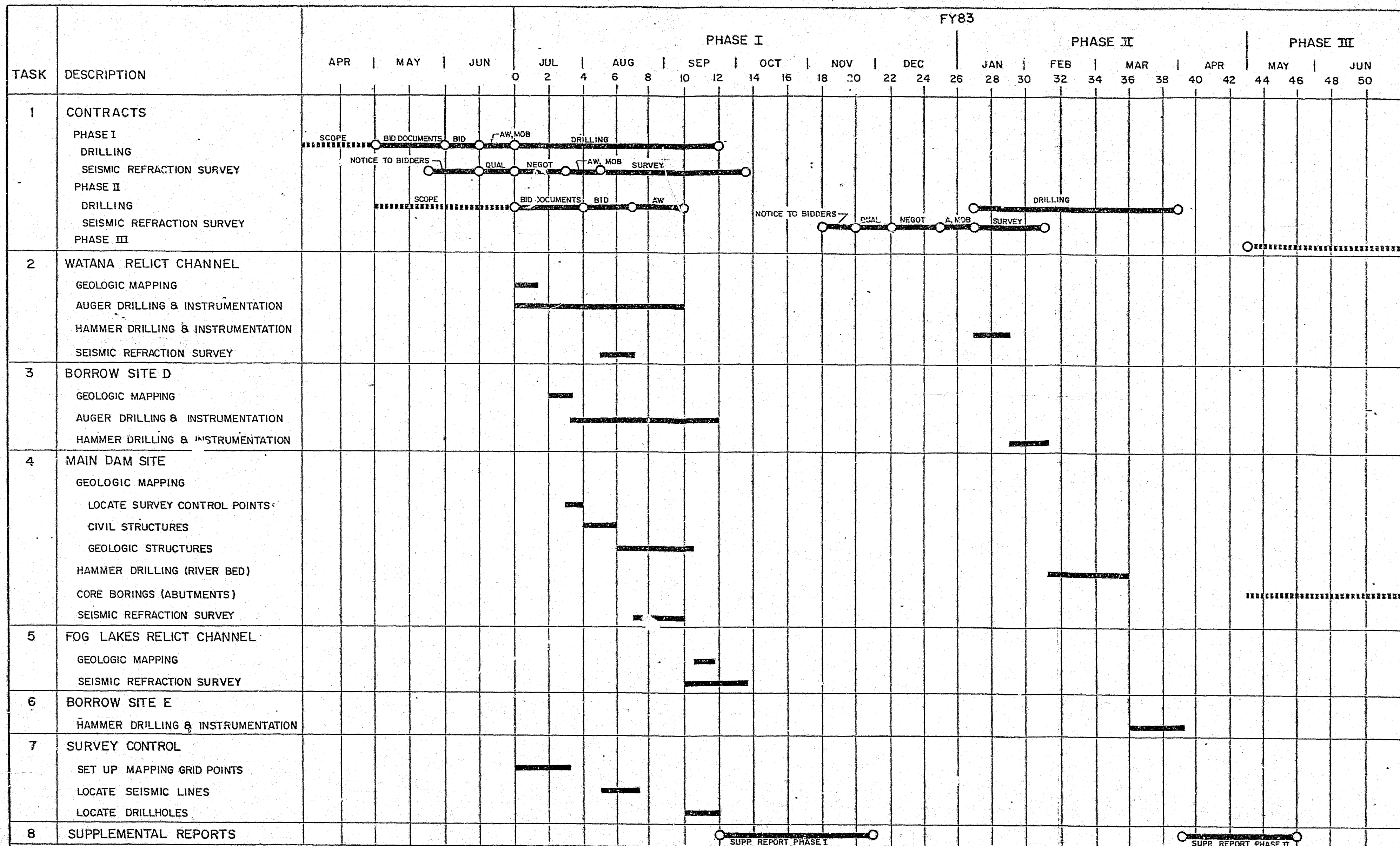
As previously specified, the majority of data reduction for the FY83 program will be performed in the field. This will allow use of the data in scoping the subsequent phases of study. The Phase I data will be used to update and refine those geologic and geotechnical interpretations presented in the Feasibility Report and the 1980-81 Geotechnical Report. As additional data becomes available, addendums to these reports, reflecting this new data, will be prepared as required. Finalization of those addenda are not anticipated until November.

**TABLE 7.1: ESTIMATED FY83 FIELD LABORATORY TESTING**

Source	No. of Holes	Gradation	Sample Type		Gradation	Testing				Retain for Future Testing
			Drive/Core	Cyclone		Hydrometer	Moisture	Atterberg	Proctor Series (Model & Stan- dard)	
Summer 1982:										
Fog Lakes Relict Channel	0	15	0	0	15	15	5	5	0	15
Watana Relict Channel & Borrow Site	24	10	800	0	200	50	50	50	0	100
	26	20	850	0	200	100	100	100	5	100
Winter 1982-83:										
Damsite River Alluvium	20	-	100	220	200	20	0	20	5	50
Watana Relict Channel & Borrow Site	9	0	200	300	150	25	50	50	5	100
	5	0	150	300	150	25	150	150	20	100
Borrow Site E	16	100	300	200	20	0	20	20	20	150
TOTAL (13,850 LF of drilling)	100	45	2,200	1,120	1,015	255	355	395	55	615

## 8 - PROJECT SCHEDULE

Figure 8.1 is the proposed schedule for FY83 Geotechnical Program. Key milestone dates to ensure performance of the work specified in Sections 2 through 5 are the mobilization of drilling equipment and personnel for Phase I into the field by July 6. Similarly, the mobilization of the hammer drill overland during December will be critical to allow for drilling in the relict channel and dam foundation. Therefore, to accomplish this, a contract for Phase I work must be in place by the 2nd to 3rd week of June. The contract for the hammer drill must be let by September to allow adequate time for mobilization of the equipment to the site.



GEOTECHNICAL SCHEDULE  
TASK V FY 83